



Ontwikkelingsmaatschappij Fortuinlijk Wonen BV (OFW)

Donaustraaf 176 8226 LC Lelystad Nederland

Tender Document	Timber frame construction
Project	School building, 2 storey, for 160 students
Client	MFI International, Holland Station 24, 6063 NP Vlodrop, The Netherlands
Project Management	OFW, Station 24, 6063 NP Vlodrop, The Netherlands Christian Schweizer, independent architect and city planner Station 24, 6063 NP Vlodrop, Nederland Tel. +31 475538539
Architect	Alberto Castaño, freischaffender Architekt MERU, Station 24, 6063 NP Vlodrop, Nederland sthapatyaveda@web.de Tel. +31 475538541
Building sites	Austria, Belgium, Bosnia-Herzegovina, Bulgaria, Denmark, Greece, Hungary, Latvia, Portugal, Serbia and Sweden
Date	May 29, 2007

Deadline for offers

Likely date start of work

Proposed finishing date

June 2007

December 2007



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Content

1. Tender Document and building description
2. Architectural drawings scale 1/100
3. Detail drawings scale 1//20 and 1/10
4. Preliminary structural calculation
5. Preliminary heat protection and energy balance calculation



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Project data and design description

Private school with 8 classrooms in first and second floor, 20 students each, total 160 students

Recreation and gymnastic halls in the ground floor (entrance floor)

The main building material will be insulated wooden timber frame cassettes

Rectangular footprint of a 3 storey building L/W = 33,18 / 12,78 m, H = 11,82 m

Façade modular system 1,70 m

1° Inclined roof with bitumen covering

Arched windows, 0,90 / 1,80 m

Load capacity of the existing soil: 200 KN/m² (assumed)

Fire protection

The central exhibition hall and the stair cases are supposed to be the main “fire compartment” and should be built in the quality F90B (German fire protection standards), including surrounding walls, ceilings, doors, glass panels and stairs, inclusive of all fittings, handles, smog window / smoke detector etc.

Requirements and basis for price calculations

Turn key offer for buildings ready for immediate use

The offer should be cost-free

Basis for calculations are the attached drawings and calculations and the present building description

The construction must certificate the basic approval of the building type and the used materials in the corresponding export country

Turn key concept components

Take over of the project, work planning and engineering

Production of the building in the local factory

Transport to the corresponding country

Erection of the elements on the locally prepared foundation

Functionality proof and hand over of the project

Contents of the tender

Description and / or pricing for the used tender components:

1. Building elements as complete work, rough and finishing's:
 - walls, ceilings, roof trusses, covering and plumbing work, dome with 2 windows, stairs, railings, windows, doors
2. Technical installations:
 - electricity, heating, ventilation, sanitary installations
3. Engineering:
 - structural calculation, energy balance certificate (European energy conservation directive), heating / ventilation power calculations and dimensioning, heat, fire and acoustical insulation proof standards, building site security plan
4. Certificate standards, like:
 - ISO 9001, RAL, etc.
5. Capacity:
 - maximal production capacity of the company in terms of “number of houses production per day”, or similar, should be mentioned in the offer
6. Export, VAT and further custom regulations:
 - information of the current situation, VAT rates etc. should be explicitly mentioned in the offer



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Material building description - premises

NATURAL MATERIALS FOR HEALTH PROMOTING BUILDINGS

In our building projects we are favoring the following materials:

WALLS

Recommended: timber, clay, bricks, porotone, liapore (or leca), natural stone
Acceptable: cellular concrete (Ytong or Siporex)
Not acceptable: steel, plastic, reinforced concrete

WALL AND ROOF INSULATION

Recommended: flax, paper wool, timber wool, wood fibber board, sheep wool,
Acceptable: mineral wool (rock wool), cellular concrete
Not acceptable: styrofoam or polystyrol

DOORS AND WINDOWS

Recommended: solid wood
Acceptable: PVC window frames, aluminium frames
Not acceptable: steel (except for fire regulations)

SURFACE MATERIALS

Recommended: marble and other natural stone, wooden pannels, plywood, gypsum board,
plaster, stucco, ceramic tiles, clay, wall paper
Acceptable: artificial marble
Not acceptable: steel, plastic, pure cement plaster

ROOF SURFACE

Recommended: ceramic tiles, wooden shingles, stone slate or antracit, tar paper,
Acceptable: cement tiles, zinc and copper plate in small amount
Not acceptable: steel sheet

PAINTS IMPREGNATORS AND GLUES

Recommended: natural oil paint, caseine, wax, varnish, white cement glue
Acceptable: borax salt
Not recommended: all synthetic paints and glues

ELECTRICITY

Recommended: shielded cables and soccets, grounded installation
Not acceptable: non-shielded cables and nongrounded intallations

WATER PIPES

Recommended: copper pipes, composite (PE + aluminium) and PP-pipes
Acceptable: other non-toxic and cecyclable plastic pipes

BEARING STRUCTURE AND REINFORCEMENT

Recommended: mixed glass or carbon fibber cement, glassfibber rod reinforcement.
Steel structures and steel reinforcement should be avoided wherever possible.
Acceptable only in exceptional situations.



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Material building description – rough work

Plinth

Build out of the concrete foundation (plastered) and the outer door base with wooden wool panels to build the designed profiles acc. to elevation and section drawings

Outer Walls

Timber frame cassettes containing: inner gypsum board on OSB board, incl. inner installation space, 140 mm studs (i.e. German KVH-wood), mineral wool insulation, outer wooden wool panels (i.e. Heraklith) as bearing panel for plastering work, installed upon bituminous isolation strips on concrete slab. Outer plaster in two layers total thickness 2,5 cm.

Entablature profiles and facia boards

With wooden wool panels to build the designed profiles acc. to elevation and section drawings incl. sealing and coating to exterior walls

Inner Walls

Both sides gypsum board on OSB board on 65 or 80 mm studs, acc. to wall thickness (i.e. German KVH-wood), mineral wool insulation acc. to fire protection regulations, incl. installation pipes and bituminous isolation strips on concrete slab

Roof

Roof trusses in drayed, untreated composite wooden trusses (i.e. German KVH-wood) resting on outer walls and inner wooden laminated beams, incl. inner wind protection foil as vapor barrier (Tyvek foil, bitumen layer o.s.), mineral wool insulation with inclined upper side, etc. Plumbing in zinc work incl. all gutters, vertical pipes and overflows. Dome in wooden construction and zinc work and pedestal for crowning element (ornament will be supplied by client)

Stairs

In wood construction (or acc. To local regulations), with wooden handrail (F90B) construction, bottom side in exposed quality, top side prepared for laying of linoleum floor coverings, handrail in corresponding construction, all acc. to fire protection regulations.

Ceilings

Timber frame cassettes containing: downsides double gypsum board on acoustical metal profile, wood construction of 240 mm beams (or acc. to static) + mineral wool insulation, acc. to fire protection regulations, upper OSB board for the flooring work.



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Material building description - Finishing's

Windows

Wooden windows, white painted, single opening wings or double openings (arch windows), double glazing, minimum U-value 1.1 W/m²K, glass panes: design according to elevation and detail BD3, inclusive of all fittings, handles in brass, to be chosen by client. Roller shutter casing elements for sun protection incorporated in the wall.

Outer anti-fall guardrails for windows

Handrail in 90 cm height over floor level, in galvanized steel, like in detail drawing BD2.

Window sills

External aluminum window sills

Internal coated wood window sills

Doors

Wooden doors, painted white, one opening wing, double glazing, inclusive of all fittings, handles in brass, to be chosen by client

Flooring ground floor

Horizontal bituminous isolation on existing concrete slab, 10 cm mineral floor insulation plates, Pe-Foil 0,5mm, 70mm floating screed,

Flooring other floors

50mm floating floor out of gypsum board on wooden fiber board on OSB board, mineral wool insulation between the beams.

Natural Linoleum floor

On all floors except rest rooms, incl. all extra work and connections, skirting boards, incl. all necessary grouting

Painting

Plastering and painting of inner and outer side of exterior walls and of inner wall, as complete work, incl. all necessary pre-treatments, all connections and expansion joints, Natural color on water basis, pigment to be chosen by the client.

Dividing walls for bathrooms

Height 2300 mm, incl. 150 mm air space at foot level, pass line height 2000 mm, incl. all doors, ceramic plates and tiles with a coated surface incl. cup board.



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Description technical Installations

Sanitary

Complete sanitary installation

For 5 WC units, 2 pissoirs, 5 sinks, incl. all pressure pipes, supplier's service installation (water meter). Quality of the chosen materials to be of medium standard, incl. 3 mirrors 100x50cm

Heating

Complete heating system

Central gas-fired low temp. heating system ca. 30 KW, incl. all pressure pipes, supplier's service installation (gas meter). Quality of radiators and other chosen materials to be of medium standard.

Central hot processed water production storage boiler with heat protection device, heating pipe circuits, supplier's service installation (gas meter) according to calculation

Radiator with flat valve and pipe connections, white colour

Electrical

Complete electrical installation

Equipment with shielded cables, including grounding and lightning conductors, electric installation and supplier's service installation (electricity meter).

Quality of the chosen materials to be of medium standard.

Lighting

- 32 ceiling lamps for classrooms
- 6 ceiling lamps for exhibition halls
- 1 candelabra hanging down in the center (dome)
- 16 ceiling lamps for exercise halls
- 4 ceiling lamps for teachers room
- 6 ceiling lamps for WC
- 4 ceiling lamp for the entrance hall
- 4 ceiling lamps for library room
- 6 ceiling lamps for computer rooms

Switches

55 combinations switch and socket, 13 single sockets, including covering plates,

Lighting: one outdoor lamp in each window arch, 6 spots under porch roof. Quality of the chosen lamps medium standard and

Ventilation

Consumed-air-extracting ventilation system, with heat exchanger (heat devolution for energy savings), no airconditioning, but just gradual air extracting system.



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**Tender Document
Project**

Timber frame construction
School building 2-storey for 200 students

Name of the company.....

Address of the company.....

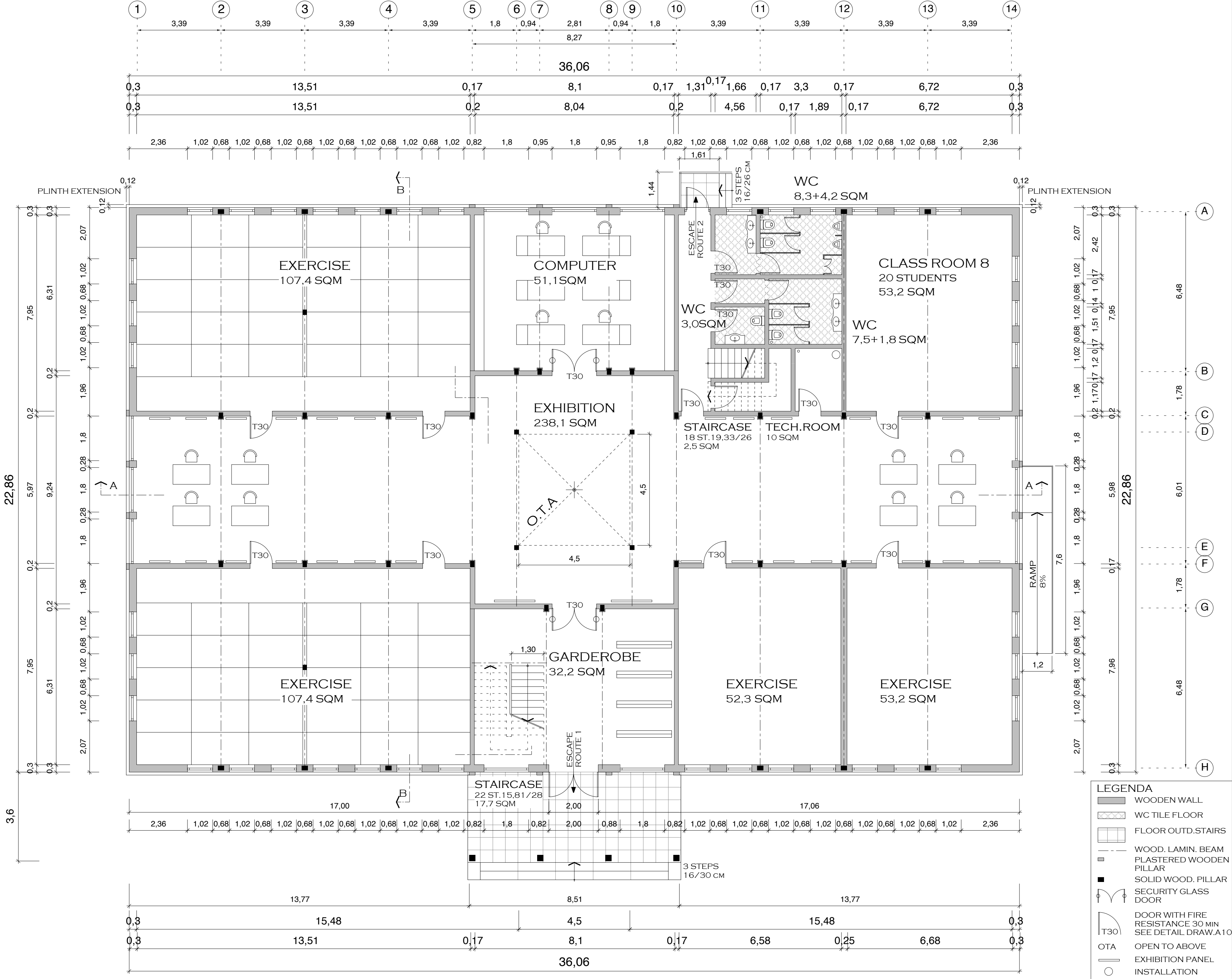
Total net price for 1 school building..... Euro

Total gross price for 1 school building..... Euro

Included VAT.....% (if applicable)..... Euro

Location..... **Date**

Signature.....





MAHARISHI INVINCIBILITY SCHOOLS

LOCATION

CLIENT
ONTWIKKELINGSMAATSCHAPPIJ FORTUINLIJK WONEN BV. (OFW)
DONAUSTRAT 176
8226 LC LELYSTAD
NEDERLAND

PROJECT MANAGER
FREIER ARCHITEKT+STADTPLANER
CRISTIAN SCHWEIZER
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PROJECT ARCHITECT
ALBERTO CASTAÑO
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BUILDING COMPANY

TEL.

BUILDING SITE MANAGER

TEL.

STRUCTURAL ENGINEERING

TEL.

OFFICIAL SURVEYOR

TEL.

HOUSE TECHN. ENGINEERING

TEL.

PARAMETERS

FFL-GF	0,00 M
-FINISH FLOOR LEVEL GROUND FLOOR-	
FFL-FF	+3,48 M
-FINISH FLOOR LEVEL FIRST FLOOR-	
TERRAIN	-0,48 M
-FINISH FLOOR LEVEL SOIL-	
FFL-W	+4,38 M
-FINISH FLOOR LEVEL WINDOW-	
FLAT ROOF SLOPE	CA. 1°
MODULE	M
OUTER WALL THICKNESS	0,30 M
CLASS ROOM WALL TH.	0,25 M
CEILING THICKNESS	0,42 M

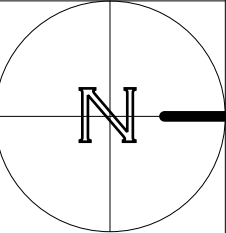
ALL DIMENSIONS TO BE VERIFIED BY THE STRUCT. CALCULATIONS

PRELIMINARY PROJECT GROUND FLOOR

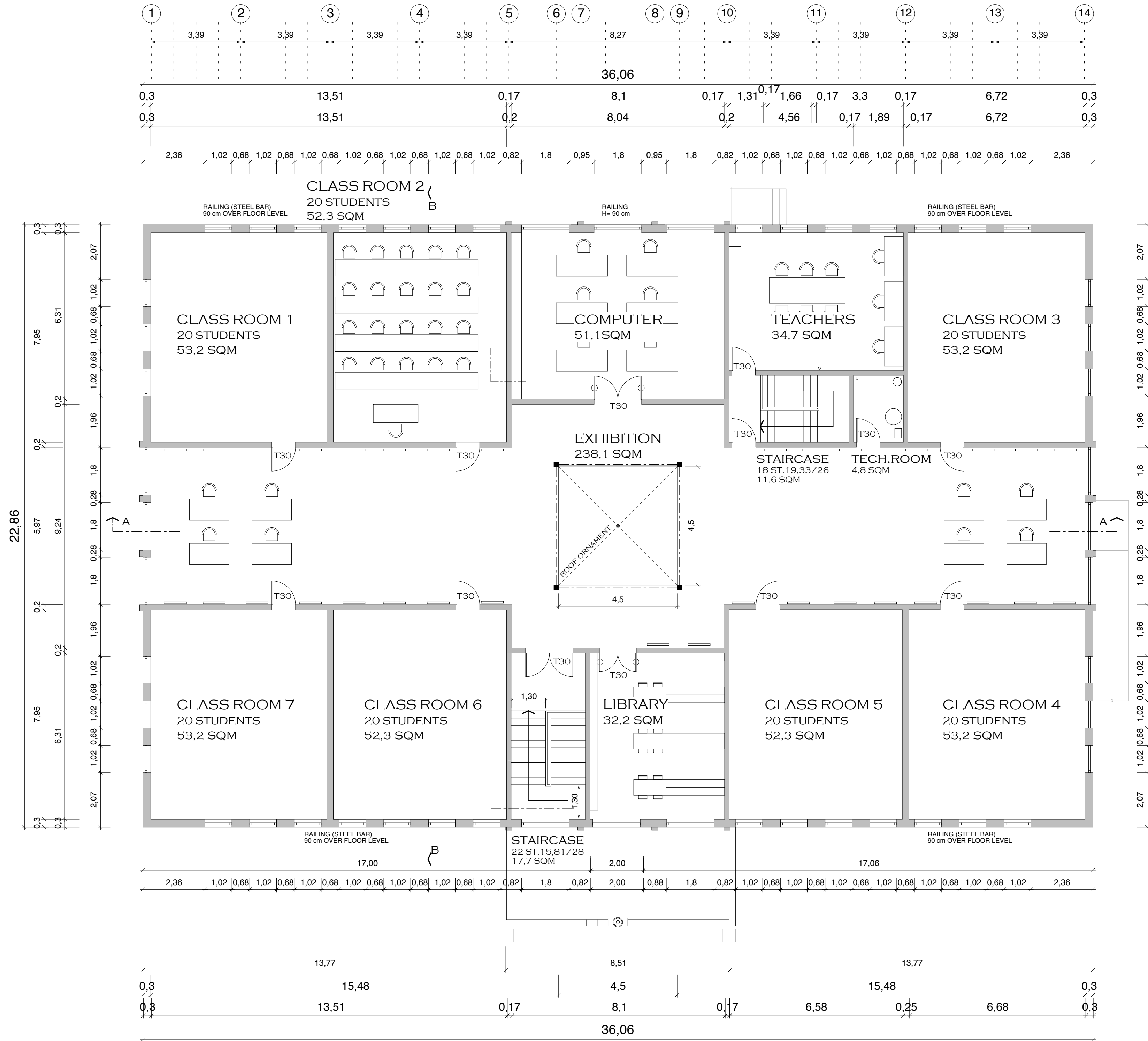
SCALE 1/100	DESIGN CA.
DATE 31.05.2007	CHANGE

DRWG. NR.

A1



- LEGENDA
- WOODEN WALL
 - WC TILE FLOOR
 - FLOOR OUTD.STAIRS
 - WOOD. LAMIN. BEAM
 - PLASTERED WOODEN PILLAR
 - SOLID WOOD. PILLAR
 - SECURITY GLASS DOOR
 - DOOR WITH FIRE RESISTANCE 30 MIN SEE DETAIL DRAW.A10
 - OTA OPEN TO ABOVE
 - EXHIBITION PANEL
 - INSTALLATION



MAHARISHI
INVINCIBILITY
SCHOOLS

LOCATION

CLIENT
ONTWIKKELINGSMAAATSCHAPPIJ
FORTUINLIJK WONEN BV. (OFW)
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FLAT ROOF SLOPE	CA. 1°
MODULE	M
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CALCULATIONS

PRELIMINARY PROJECT
FIRST FLOOR

SCALE
1/100

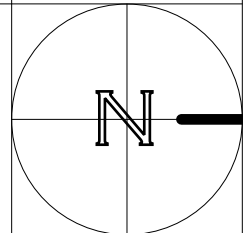
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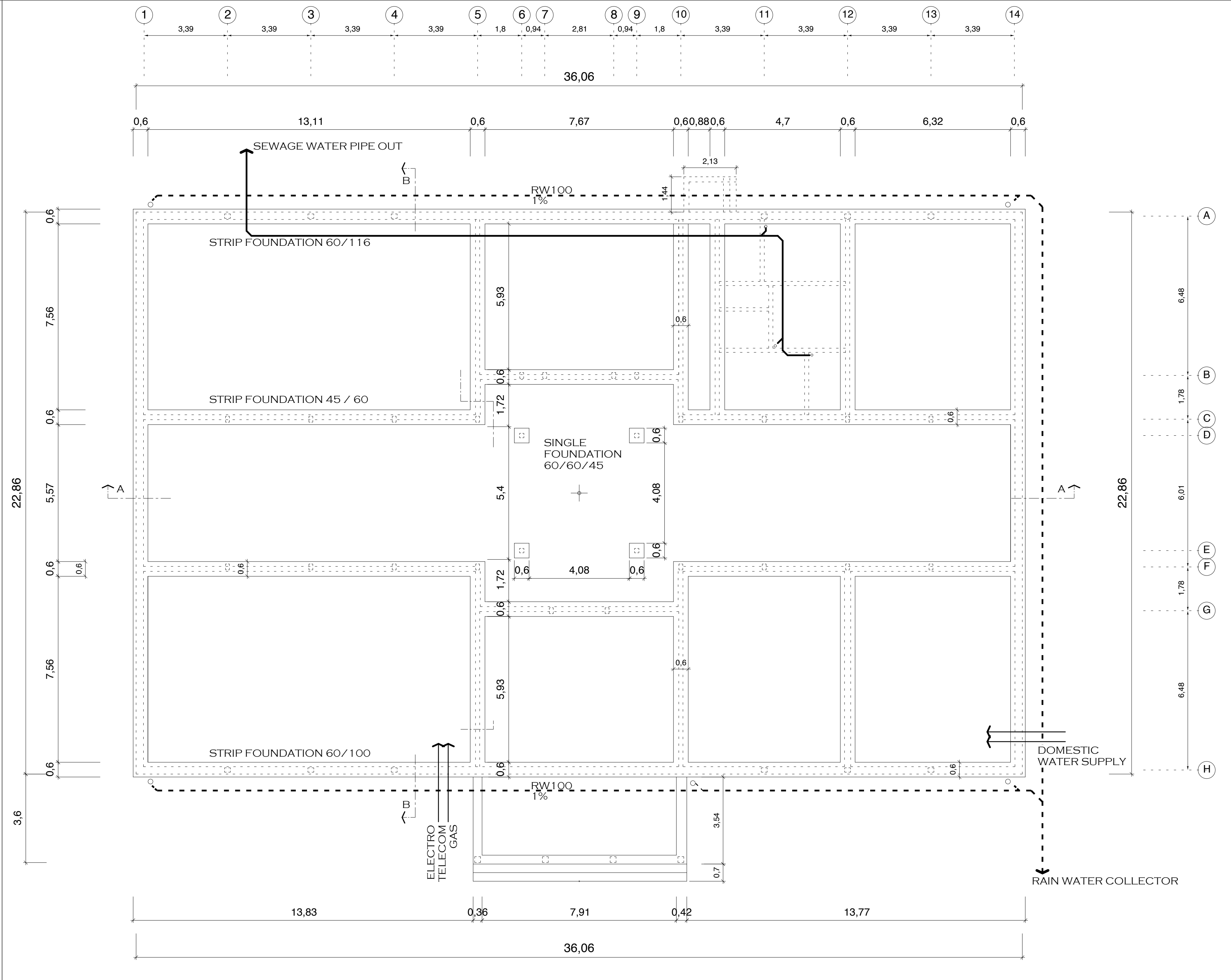
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CHANGE

DRWG. NR.

A2





MAHARISHI
INVICIBILITY
SCHOOLS

LOCATION

CLIENT
ONTWIKKELINGSMAATSCHAPPIJ
FORTUINLIJK WONEN BV. (OFW)
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LEGENDA
FFL-GF 0,00 M
·FINISH FLOOR LEVEL GROUND FLOOR-
FFL-FF +3,48 M
·FINISH FLOOR LEVEL FIRST FLOOR-
TERRAIN -0,48 M
·FINISH FLOOR LEVEL SOIL-
FFL-W +4,38 M
·FINISH FLOOR LEVEL WINDOW-
FLAT ROOF SLOPE CA.1°
MODULE M
OUTER WALL THICKNES 0,30 M
CLASS ROOM WALL TH. 0,25 M
CEILING HIGHT 0,42 M

ALL DIMENSIONS TO BE
VERIFIED BY THE STRUCT.
CALCULATIONS

PRELIMINARY PROJECT
FOUNDATION

SCALE
1/100

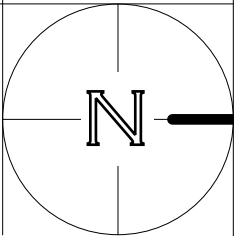
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CA.

DATE
31.05.2007

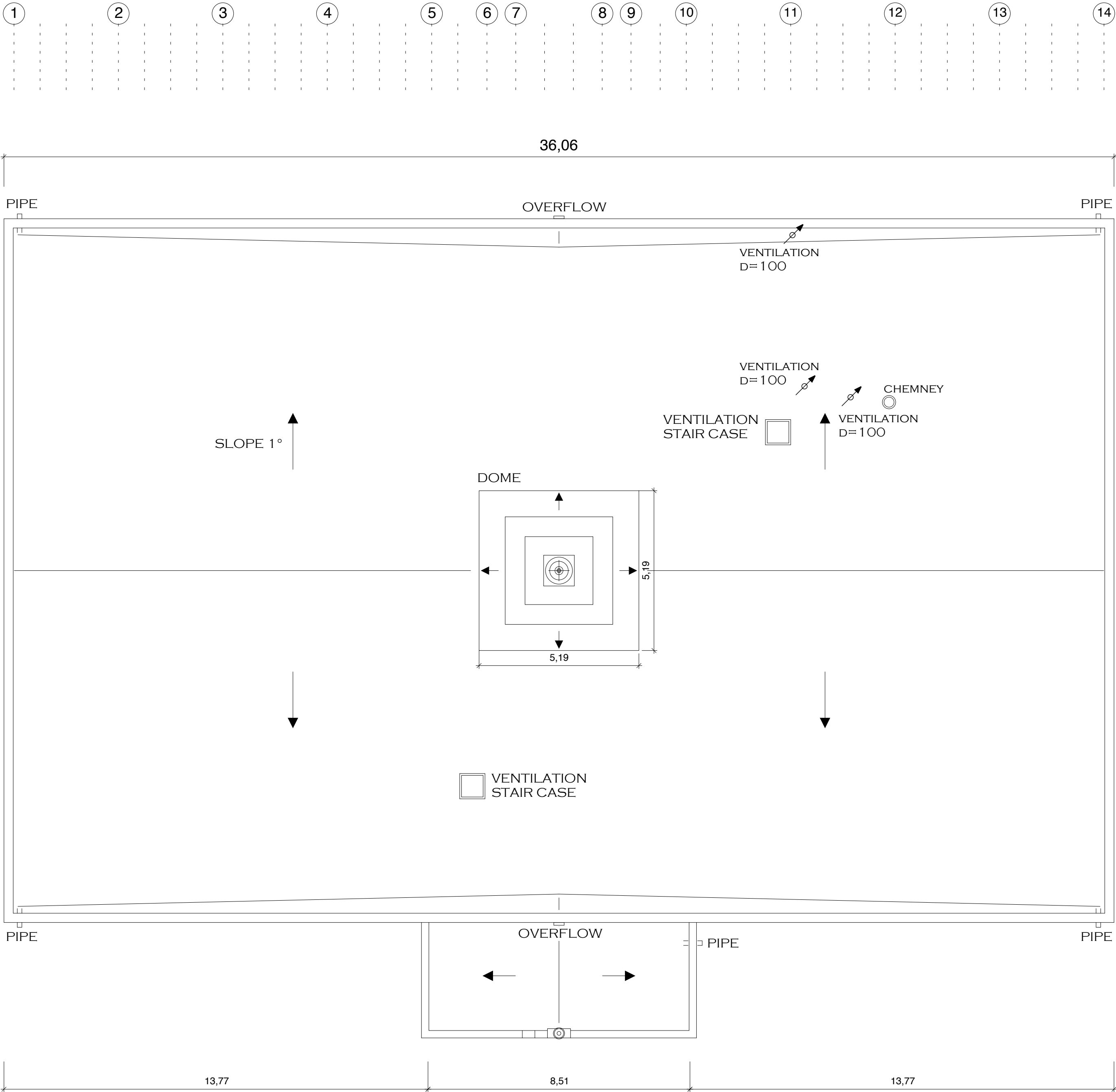
CHANGE

DRWG. NR.

A12



22,86



MAHARISHI
INVINCIBILITY
SCHOOLS

LOCATION

CLIENT
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HOUSE TECHN. ENGINEERING

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LEGENDA
FFL-GF 0,00 M
·FINISH FLOOR LEVEL GROUND FLOOR·
FFL-FF +3,48 M
·FINISH FLOOR LEVEL FIRST FLOOR·
TERRAIN -0,48 M
·FINISH FLOOR LEVEL SOIL·
FFL-W +4,38 M
·FINISH FLOOR LEVEL WINDOW·
FLAT ROOF SLOPE CA.1°
MODULE M
OUTER WALL THICKNES 0,30 M
CLASS ROOM WALL TH. 0,25 M
CEILING HIGHT 0,42 M

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CALCULATIONS

PRELIMINARY PROJECT
ROOF VIEW

SCALE
1/100

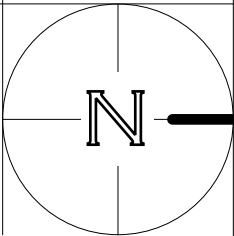
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DATE
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CHANGE

DRWG. NR.

A11





CLIENT
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BUILDING COMPANY

TEL

BUILDING SITE MANAGER

TEL

STRUCTURAL ENGINEERING

TEL

OFFICIAL SURVEYOR

TEL

HOUSE TECHN. ENGINEERING

TEL

LEGENDA	
FFL-GF	0,00 M
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FFL-FF	+3,48 M
-FINISH FLOOR LEVEL FIRST FLOOR	
TERRAIN	-0,48 M
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FLAT ROOF SLOPE	CA. 1°
MODULE	M
OUTER WALL THICKNES	0,30 M
CLASS ROOM WALL TH.	0,25 M
CEILING LIGHT	0,42 M

ALL DIMENSIONS TO BE
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CALCULATIONS

PRELIMINARY PROJECT SECTION A-A

SCALE
1/100

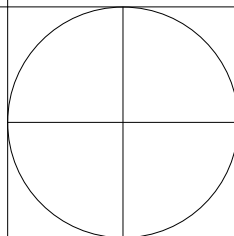
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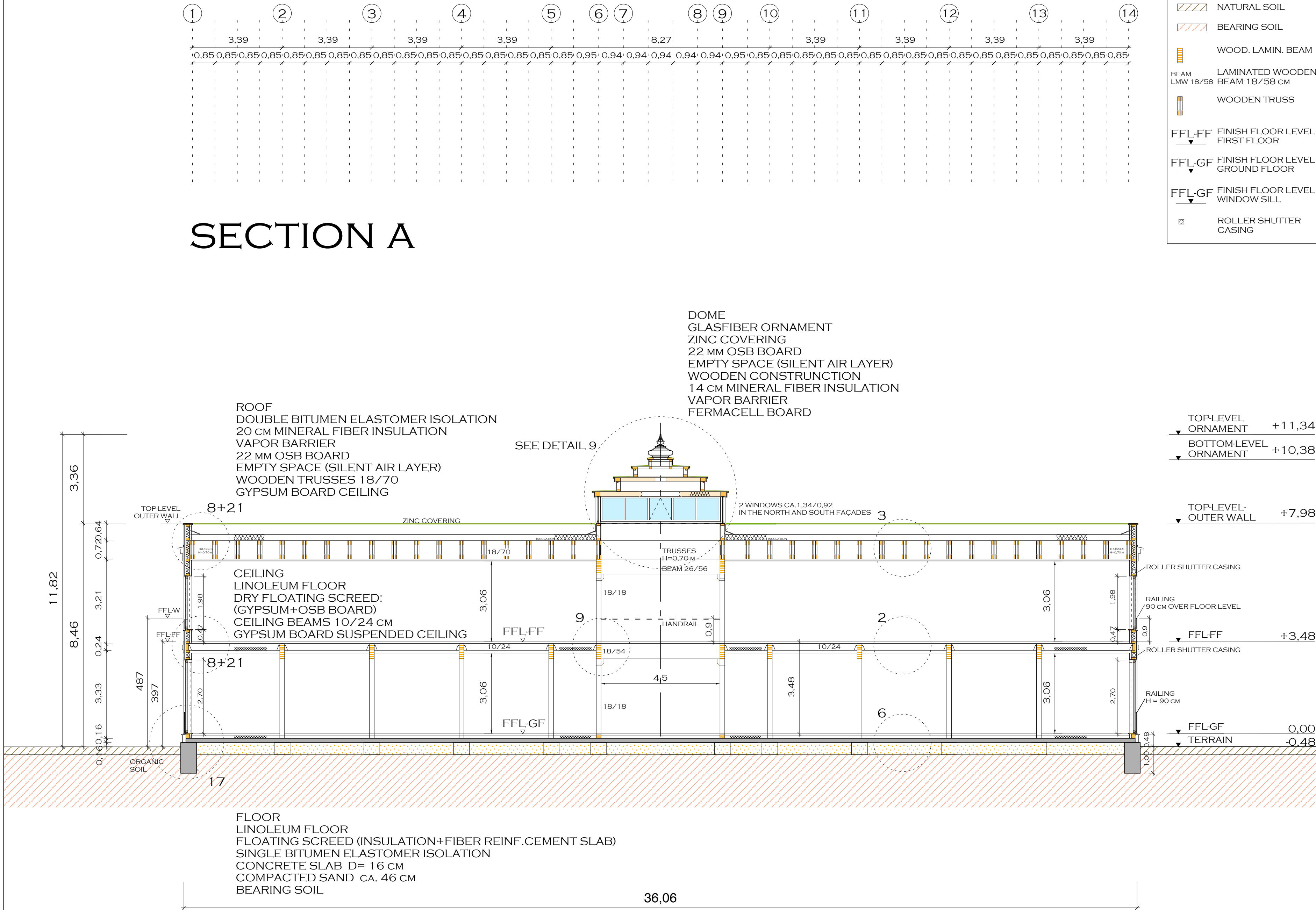
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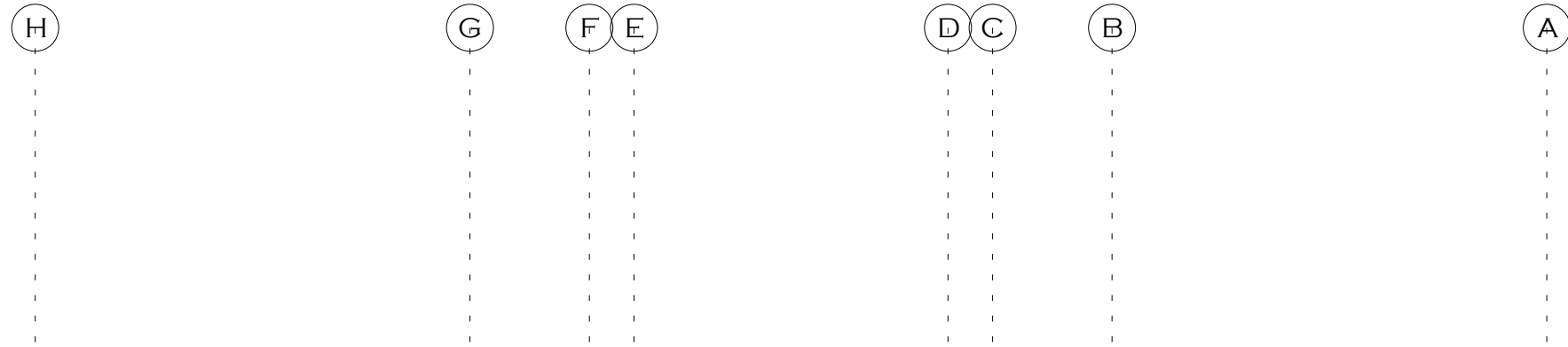
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A9



SECTION A





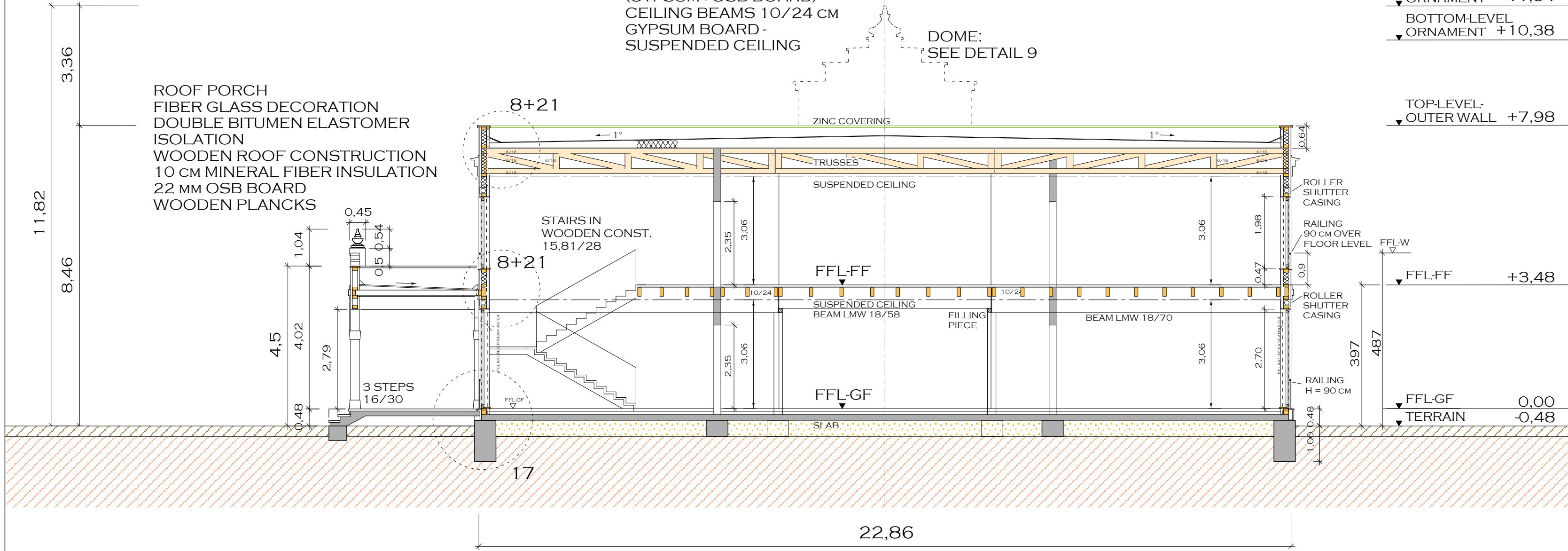
SECTION B

ROOF
 DOUBLE BITUMEN ELASTOMER ISOLATION
 20 cm MINERAL FIBER INSULATION
 VAPOR BARRIER
 22 mm OSB BOARD
 EMPTY SPACE (SILENT AIR LAYER)
 WOODEN TRUSSES 18/70
 GYPSUM BOARD CEILING

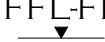
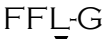
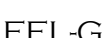
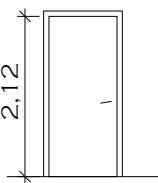
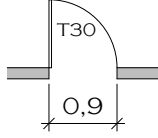
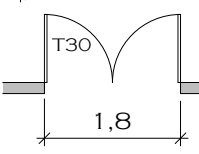
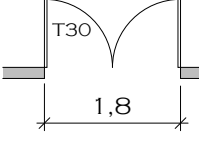
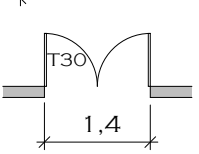
CEILING
 LINOLEUM FLOOR
 DRY FLOATING SCREED:
 (GYPSUM+OSB BOARD)
 CEILING BEAMS 10/24 cm
 GYPSUM BOARD -
 SUSPENDED CEILING

DOME:
 SEE DETAIL 9

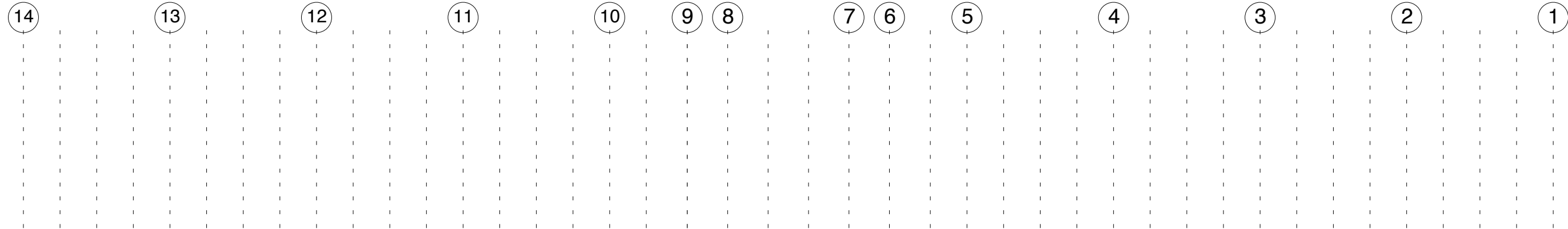
ROOF PORCH
 FIBER GLASS DECORATION
 DOUBLE BITUMEN ELASTOMER
 ISOLATION
 WOODEN ROOF CONSTRUCTION
 10 cm MINERAL FIBER INSULATION
 22 mm OSB BOARD
 WOODEN PLANCKS



FLOOR
 LINOLEUM FLOOR
 FLOATING SCREED (INSULATION+FIBER REINF.CEMENT SLAB)
 SINGLE BITUMEN ELASTOMER ISOLATION
 CONCRETE SLAB D= 16 cm
 COMPACTED SAND ca. 46 cm
 BEARING SOIL

LEGENDA		 MAHARISHI INVISIBILITY SCHOOLS	
 CONCRETE ELEMENT	LOCATION		
 COMPACTED SAND			
 NATURAL SOIL			
 BEARING SOIL			
 WOOD. LAMIN. BEAM			
 BEAM LMW 18/58	LAMINATED WOODEN BEAM 18/58 CM		
	WOODEN TRUSS		
 FFL-FF	FINISH FLOOR LEVEL FIRST FLOOR		
 FFL-GF	FINISH FLOOR LEVEL GROUND FLOOR		
 FFL-GF	FINISH FLOOR LEVEL WINDOW SILL		
	ROLLER SHUTTER CASING		
INNER DOORS		PROJECT ARCHITECT ALBERTO CASTAÑO FREISCHAFFENDER ARCHITEKT WALDSTRASSE 20 C 23911 MUSTIN TEL. 0049 4546 891170 TEL. 0031 475 538541	
	CLASS ROOM DOOR: PROPOSED DIMS. 0.90/2.12 M OR ACCORDING TO LOCAL REGULATIONS		
	MATERIAL: WOOD INCL. WOODEN FRAME		
STAIRCASE DOOR: PROPOSED DIMS. 1.80/2.12 M OR ACCORDING TO LOCAL REGULATIONS		BUILDING COMPANY	
	MATERIAL: WOOD INCL. WOODEN FRAME GLASS FIELDS LIKE IN THE DRAW.		
COMPUTER RO. DOOR: PROPOSED DIMS. 1.80/2.12 M OR ACCORDING TO LOCAL REGULATIONS		TEL.	
	MATERIAL: WOOD INCL. WOODEN FRAME GLASS FIELDS LIKE IN THE DRAW.		
LIBRARY DOOR: PROPOSED DIMS. 1.40/2.12 M OR ACCORDING TO LOCAL REGULATIONS		STRUCTURAL ENGINEERING	
	MATERIAL: WOOD INCL. WOODEN FRAME GLASS FIELDS LIKE IN THE DRAW.		
LEGENDA		TEL.	
FFL-GF		0,00 M	
-FINISH FLOOR LEVEL GROUND FLOOR-		FFL-FF	
-FINISH FLOOR LEVEL FIRST FLOOR-		TERRAIN	
-FINISH FLOOR LEVEL SOIL-		FFL-W	
-FINISH FLOOR LEVEL WINDOW-		FLAT ROOF SLOPE	
MODULE		OUTER WALL THICKNES	
		CLASS ROOM WALL TH.	
		CEILING HIGHT	

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WEST

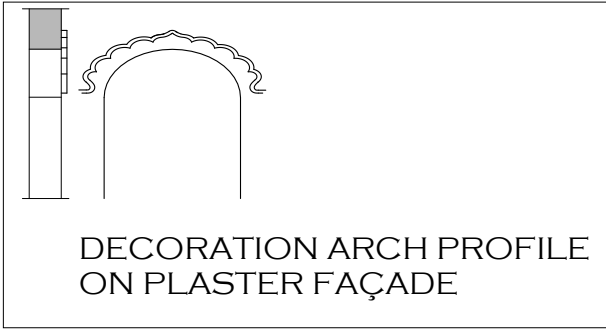
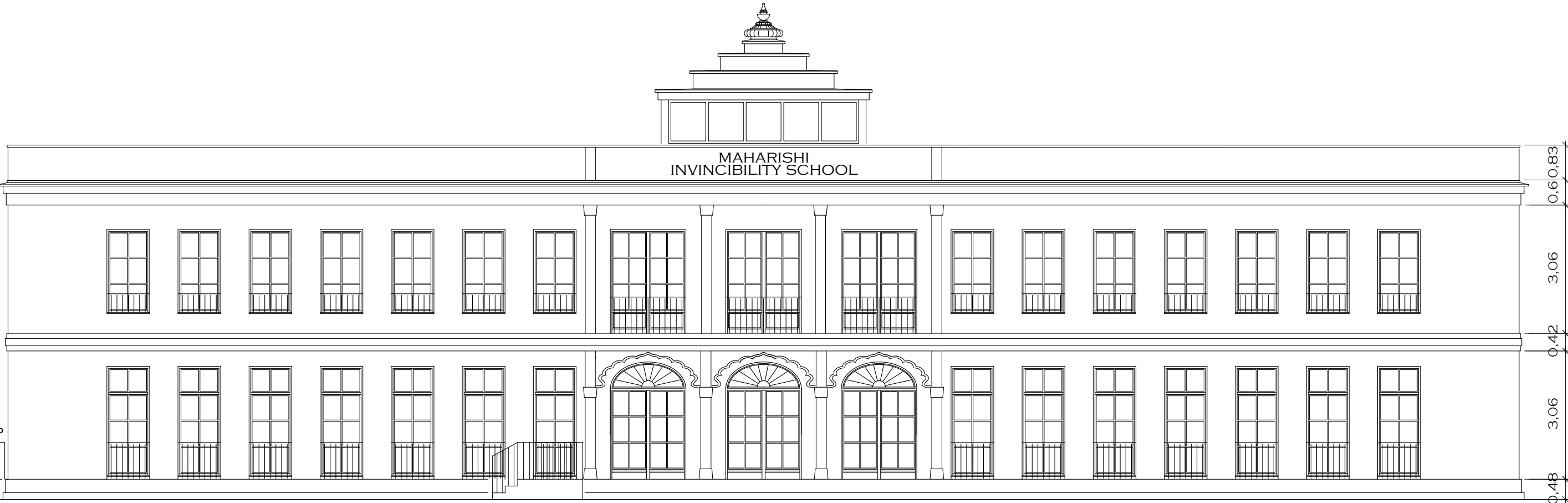
TOP-LEVEL ORNAMENT ▼ +11,34
 BOTTOM-LEVEL ORNAMENT ▼ +10,38

TOP-LEVEL-OUTER WALL ▼ +7,98

FFL-FF ▼ +3,48

FFL-GF ▼ 0,00
 TERRAIN ▼ -0,48

RAMP



DECORATION ARCH PROFILE
 ON PLASTER FAÇADE

36,06



MAHARISHI
 INVINCIBILITY
 SCHOOLS

LOCATION

CLIENT
 ONTWIKKELINGSMAATSCHAPPIJ
 FORTUINLIJK WONEN BV. (OFW)
 DONAUSTRAT 176
 8226 LC LELYSTAD
 NEDERLAND

PROJECT MANAGER
 FREIER ARCHITEKT+STADTPLANER
 CRISTIAN SCHWEIZER
 STATION 24
 6063 NP VLODROP
 NEDERLAND
 TEL. 0031 475 538539

PROJECT ARCHITECT
 ALBERTO CASTAÑO
 FREISCHAFFENDER ARCHITEKT
 WALDSTRASSE 20 C
 23911 MUSTIN
 TEL. 0049 4546 891170
 TEL. 0031 475 538541

BUILDING COMPANY

TEL.

BUILDING SITE MANAGER

TEL.

STRUCTURAL ENGINEERING

TEL.

OFFICIAL SURVEYOR

TEL.

HOUSE TECHN. ENGINEERING

TEL.

LEGENDA
 FFL-GF 0,00 M
 -FINISH FLOOR LEVEL GROUND FLOOR-
 FFL-FF +3,48 M
 -FINISH FLOOR LEVEL FIRST FLOOR-
 TERRAIN -0,48 M
 -FINISH FLOOR LEVEL SOIL-
 FFL-W +4,38 M
 -FINISH FLOOR LEVEL WINDOW-
 FLAT ROOF SLOPE CA.1°
 MODULE M
 OUTER WALL THICKNES 0,30 M
 CLASS ROOM WALL TH. 0,25 M
 CEILING HIGHT 0,42 M

ALL DIMENSIONS TO BE
 VERIFIED BY THE STRUCT.
 CALCULATIONS

PRELIMINARY PROJECT
 WEST ELEVATION

SCALE
 1/100

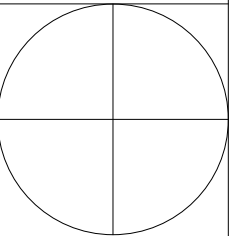
DESIGN
 CA.

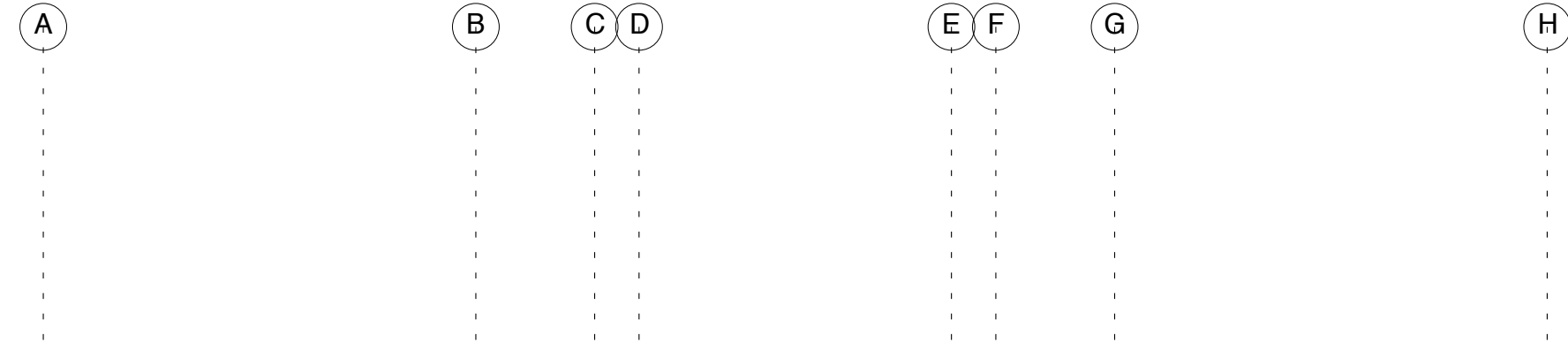
DATE
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CHANGE

DRWG. NR.

A6





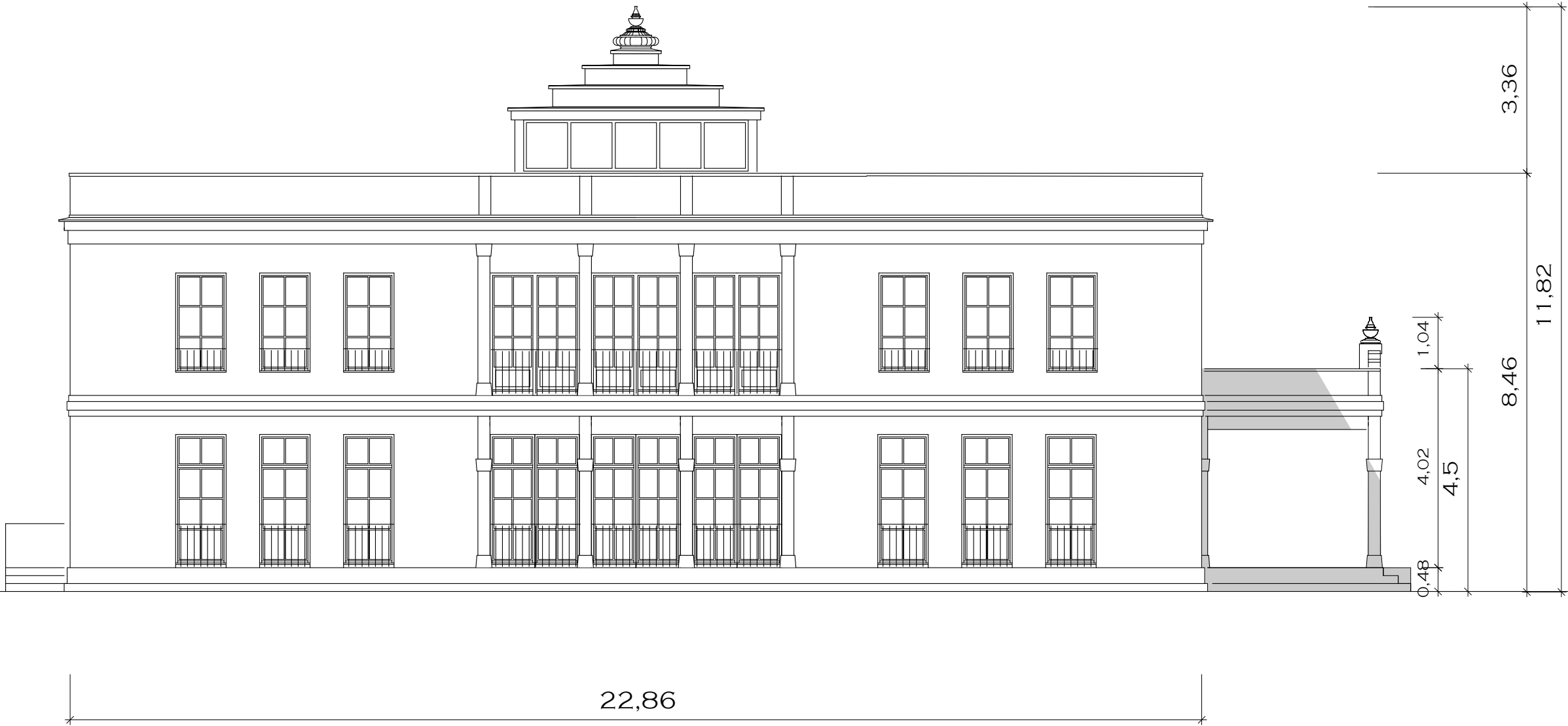
SOUTH

TOP-LEVEL ORNAMENT ∇ + 11,34
 BOTTOM-LEVEL ORNAMENT ∇ + 10,38

TOP-LEVEL- OUTER WALL ∇ + 7,98

FFL-FF ∇ + 3,48

FFL-GF ∇ 0,00
 TERRAIN ∇ -0,48



MAHARISHI
 INVINCIBILITY
 SCHOOLS

LOCATION

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 ONTWIKKELINGSMAATSCHAPPIJ
 FORTUINLIJK WONEN BV. (OFW)
 DONAUSTRAT 176
 8226 LC LELYSTAD
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HOUSE TECHN. ENGINEERING

TEL.

LEGENDA
 FFL-GF 0,00 M
 -FINISH FLOOR LEVEL GROUND FLOOR-
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 -FINISH FLOOR LEVEL FIRST FLOOR-
 TERRAIN -0,48 M
 -FINISH FLOOR LEVEL SOIL-
 FFL-W +4,38 M
 -FINISH FLOOR LEVEL WINDOW-
 FLAT ROOF SLOPE CA.1°
 MODULE M
 OUTER WALL THICKNES 0,30 M
 CLASS ROOM WALL TH. 0,25 M
 CEILING HIGHT 0,42 M

ALL DIMENSIONS TO BE
 VERIFIED BY THE STRUCT.
 CALCULATIONS

PRELIMINARY PROJECT
 SOUTH ELEVATION

SCALE
 1/100

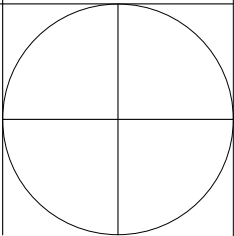
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DATE
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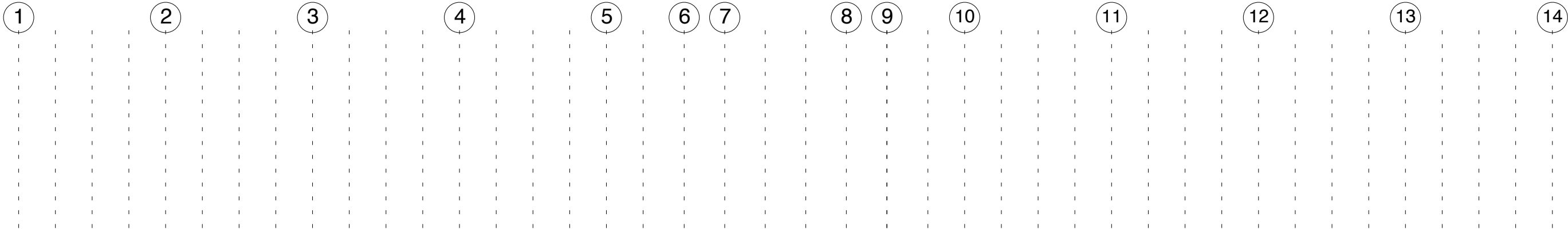
CHANGE

DRWG. NR.

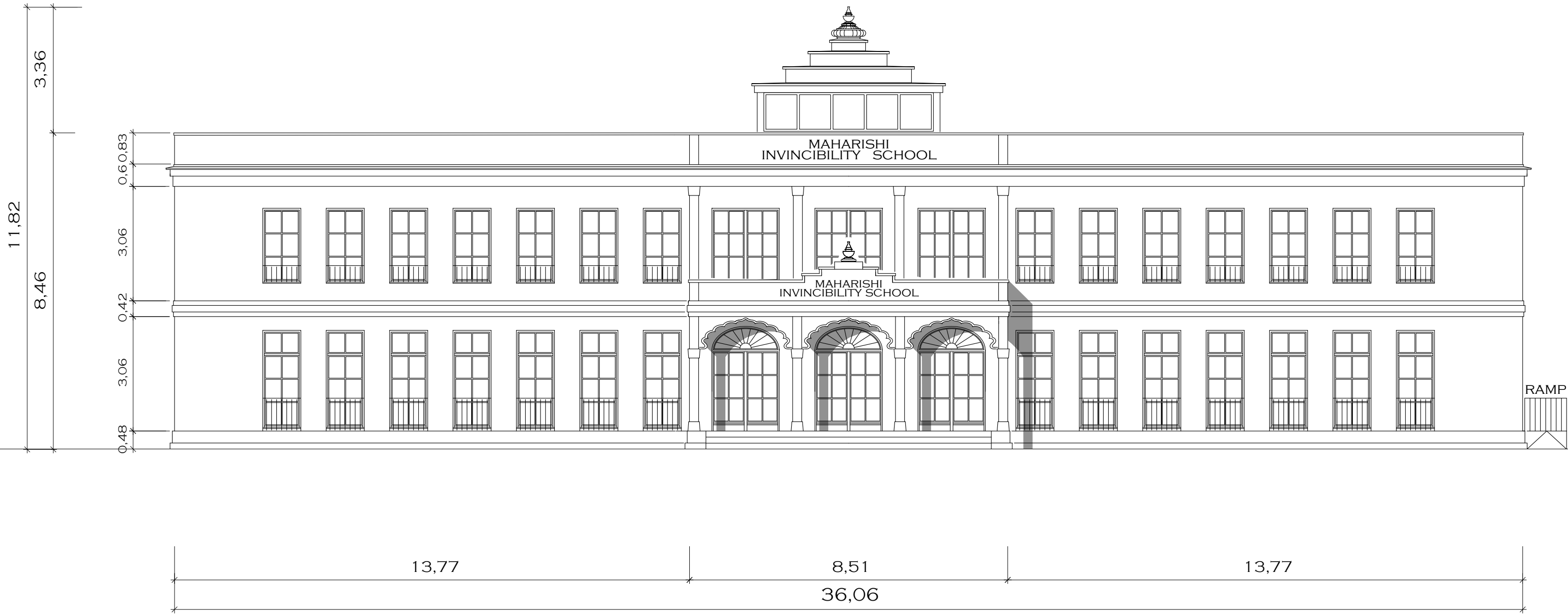
A8



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EAST



MAHARISHI
INVINCIBILITY
SCHOOLS

LOCATION

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FORTUINLIJK WONEN BV. (OFW)
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BUILDING COMPANY

TEL.

BUILDING SITE MANAGER

TEL.

STRUCTURAL ENGINEERING

TEL.

OFFICIAL SURVEYOR

TEL.

HOUSE TECHN. ENGINEERING

TEL.

LEGENDA
FFL-GF 0,00 M
·FINISH FLOOR LEVEL GROUND FLOOR-
FFL-FF +3,48 M
·FINISH FLOOR LEVEL FIRST FLOOR-
TERRAIN -0,48 M
·FINISH FLOOR LEVEL SOIL-
FFL-W +4,38 M
·FINISH FLOOR LEVEL WINDOW-
FLAT ROOF SLOPE CA.1°
MODULE M
OUTER WALL THICKNES 0,30 M
CLASS ROOM WALL TH. 0,25 M
CEILING HIGHT 0,42 M

ALL DIMENSIONS TO BE
VERIFIED BY THE STRUCT.
CALCULATIONS

PRELIMINARY PROJECT
EAST ELEVATION

SCALE
1/100

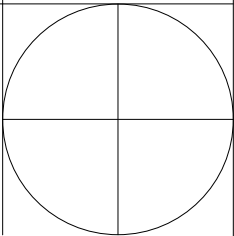
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CA.

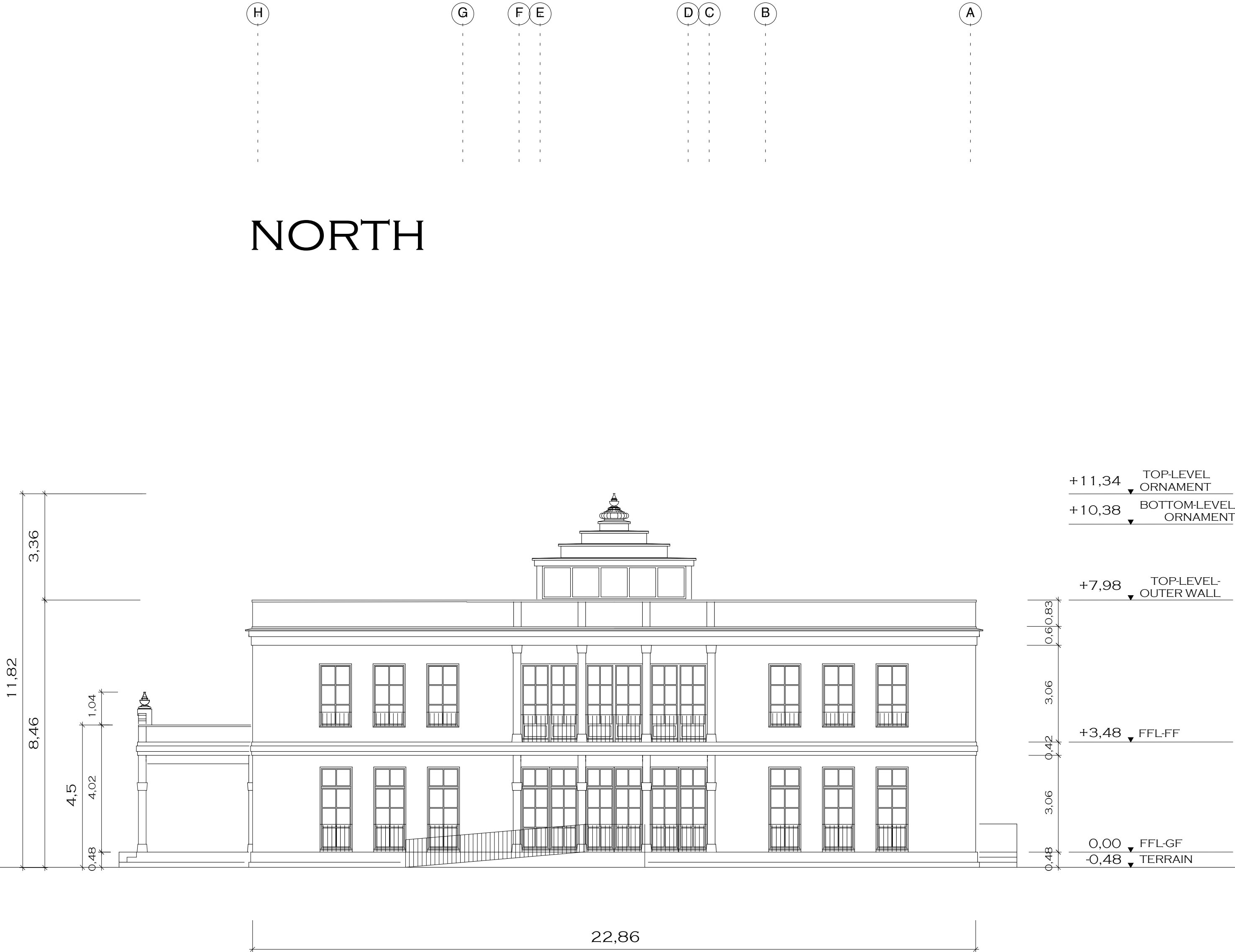
DATE
31.05.2007

CHANGE

DRWG. NR.

A5





MAHARISHI
INVINCIBILITY
SCHOOLS

LOCATION

CLIENT
ONTWIKKELINGSMAATSCHAPPIJ
FORTUINLIJK WONEN BV. (OFW)
DONAUSTRAT 176
8226 LC LELYSTAD
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HOUSE TECHN. ENGINEERING

TEL.

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FFL-FF +3,48 M
-FINISH FLOOR LEVEL FIRST FLOOR-
TERRAIN -0,48 M
-FINISH FLOOR LEVEL SOIL-
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-FINISH FLOOR LEVEL WINDOW-
FLAT ROOF SLOPE CA.1°
MODULE M
OUTER WALL THICKNES 0,30 M
CLASS ROOM WALL TH. 0,25 M
CEILING HIGHT 0,42 M

ALL DIMENSIONS TO BE
VERIFIED BY THE STRUCT.
CALCULATIONS

PRELIMINARY PROJECT
NORTH ELEVATION

SCALE
1/100

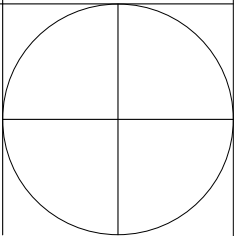
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CA.

DATE
31.05.2007

CHANGE

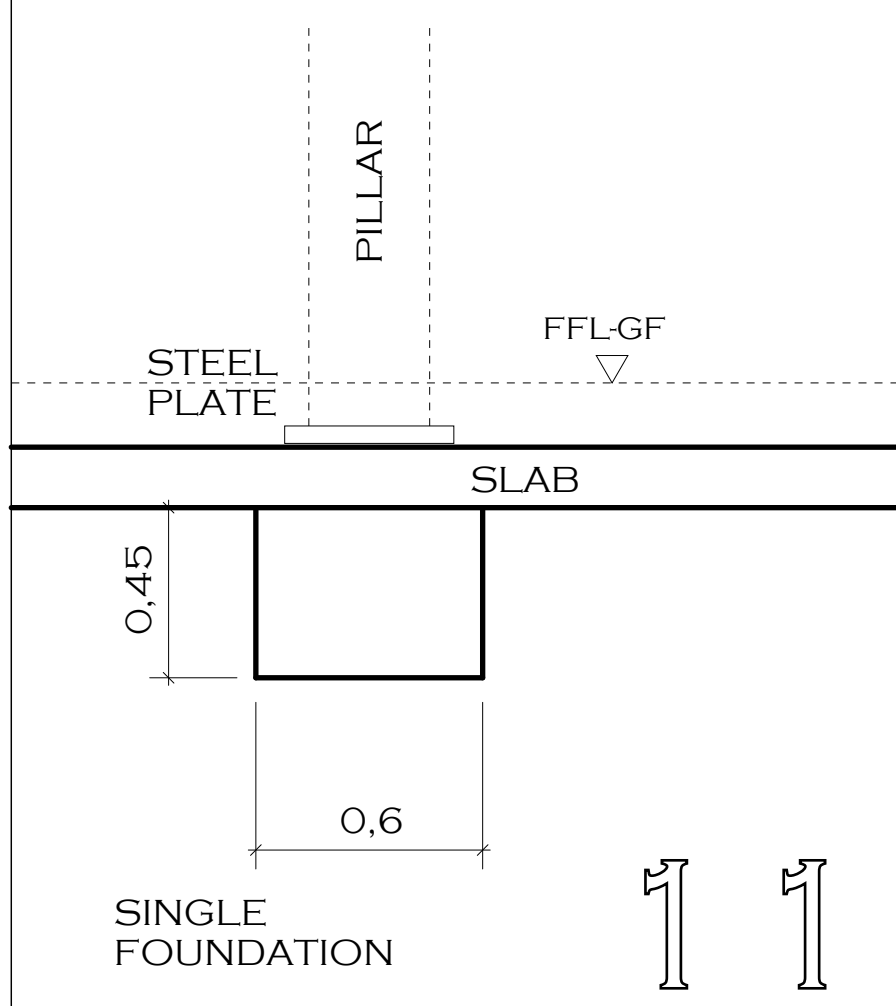
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A7

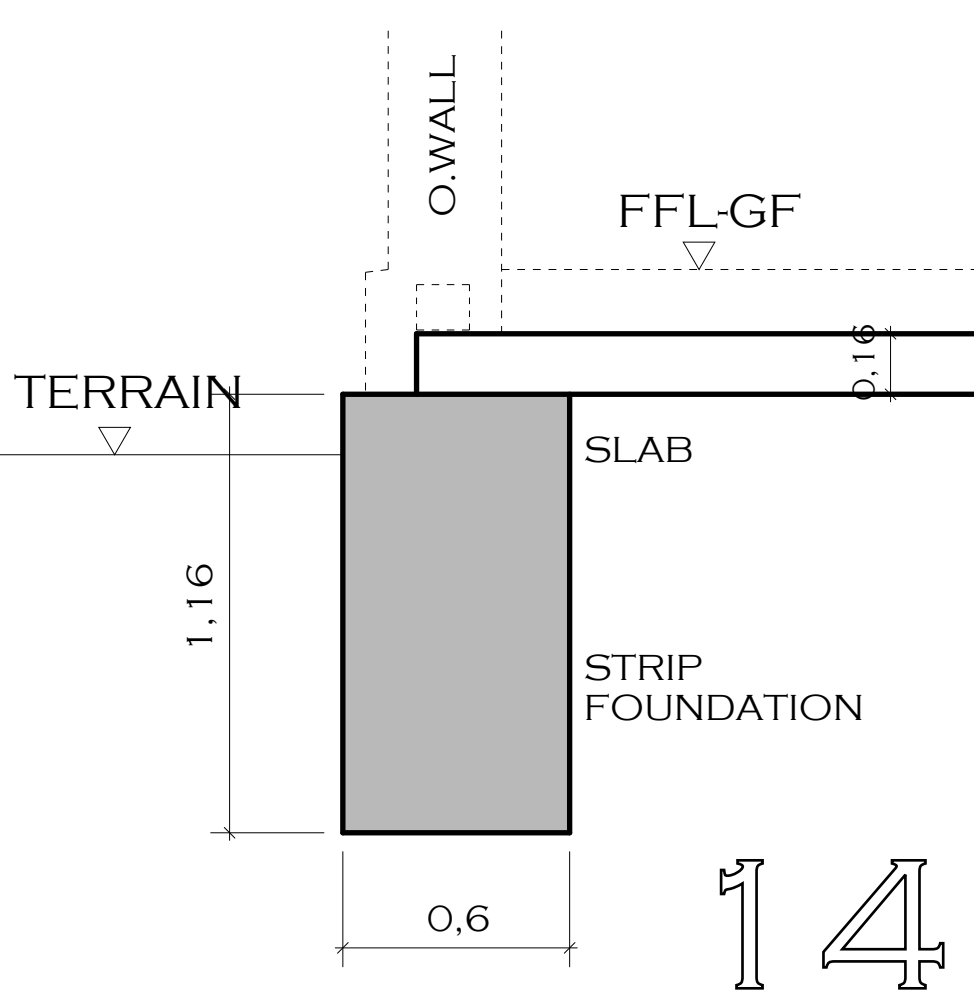


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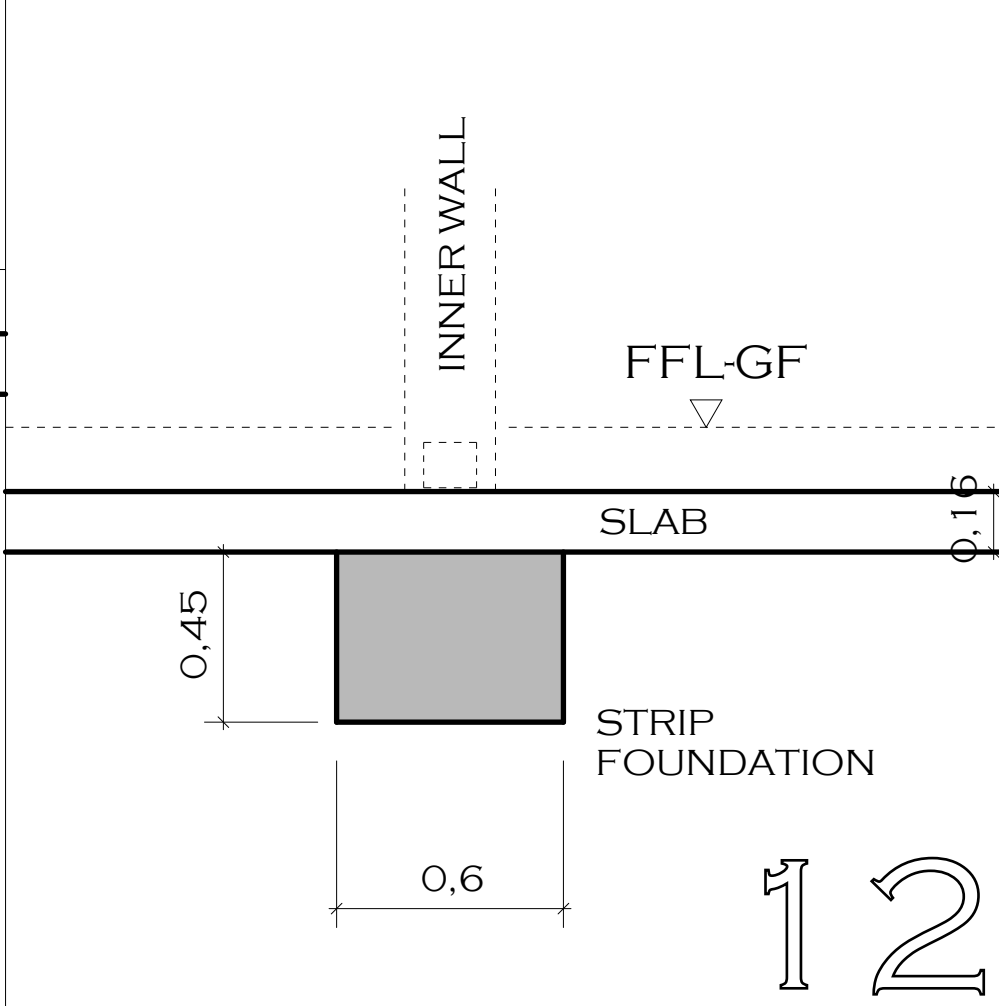
SINGLE FOUNDATION 1/20



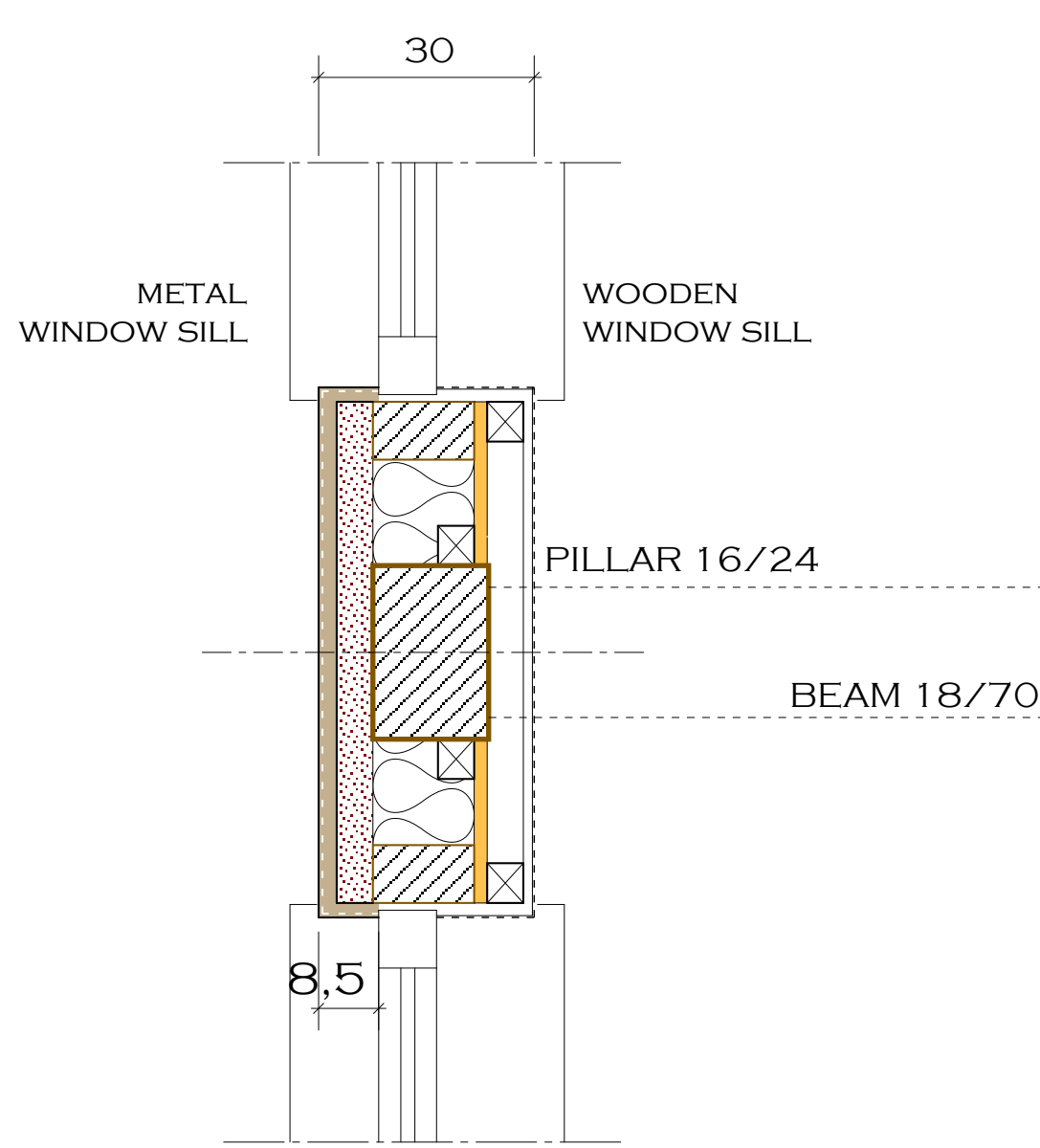
PERIMETER FOUNDATION 1/20



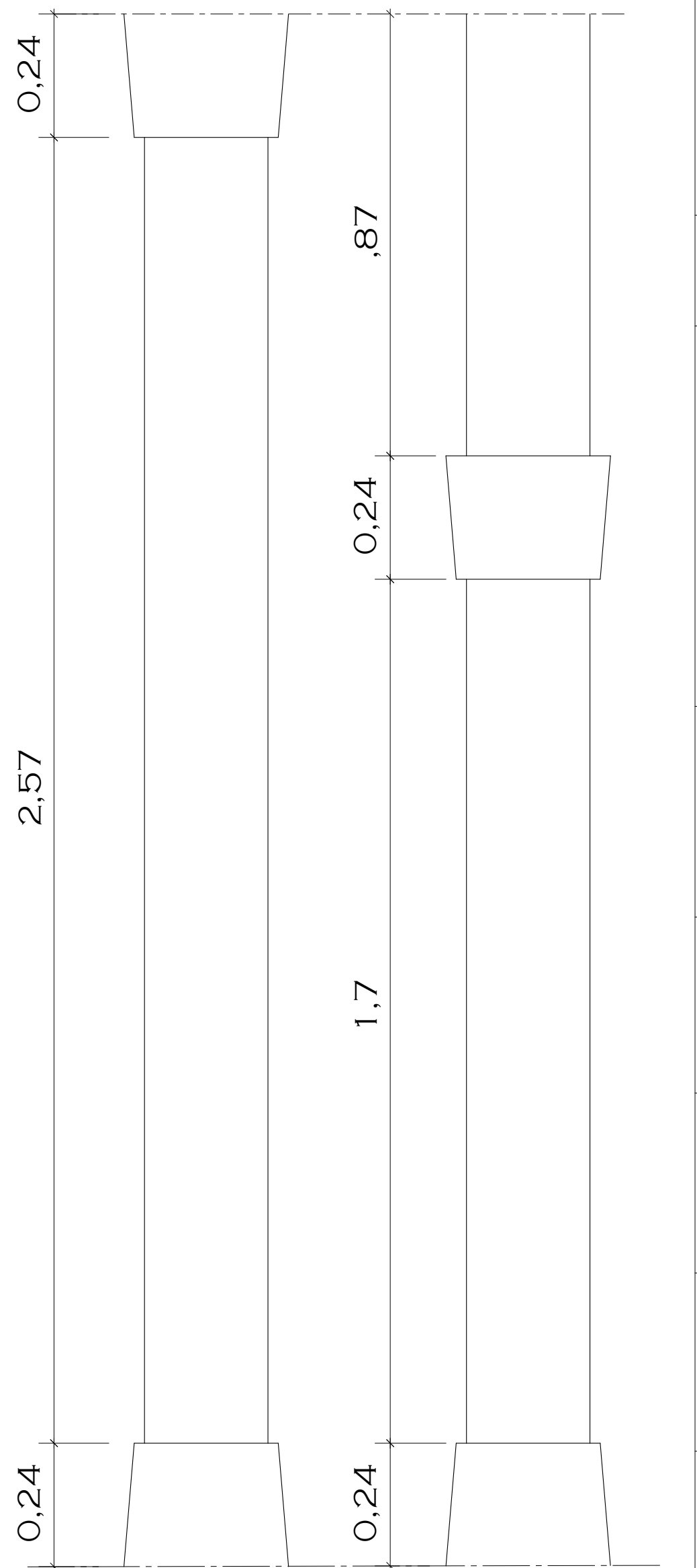
INNER WALL FOUNDATION 1/20



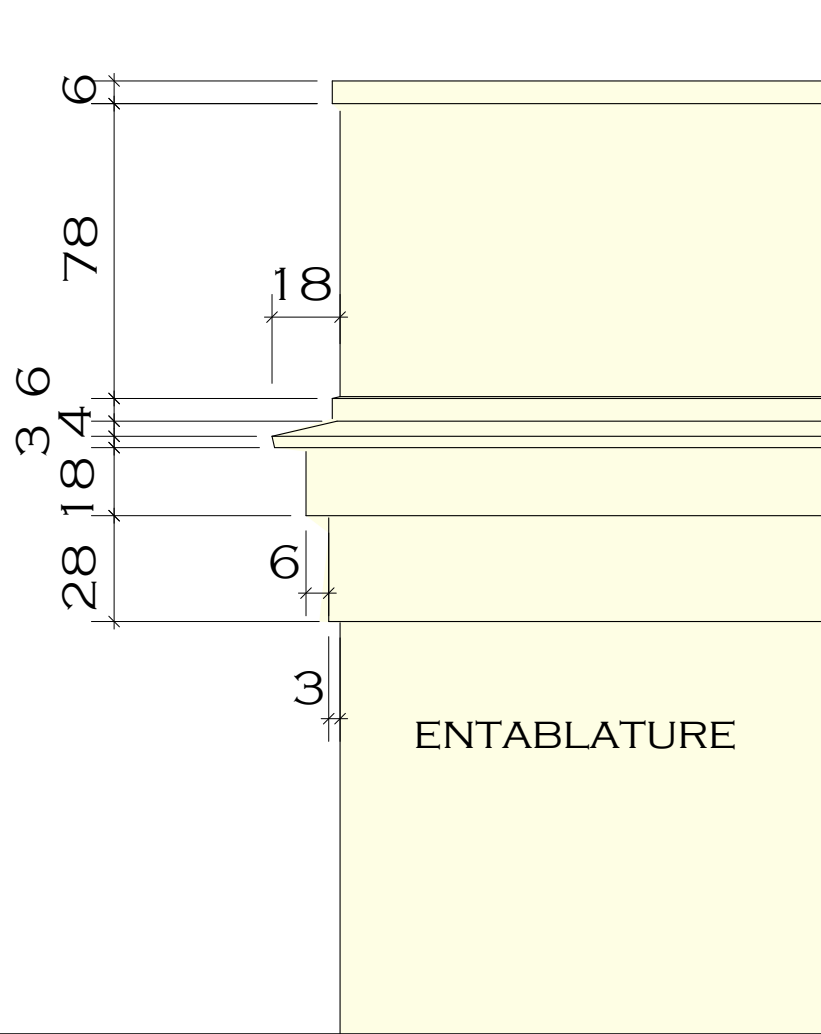
PILLAR POSITION IN PERIMETER WALL 1/10



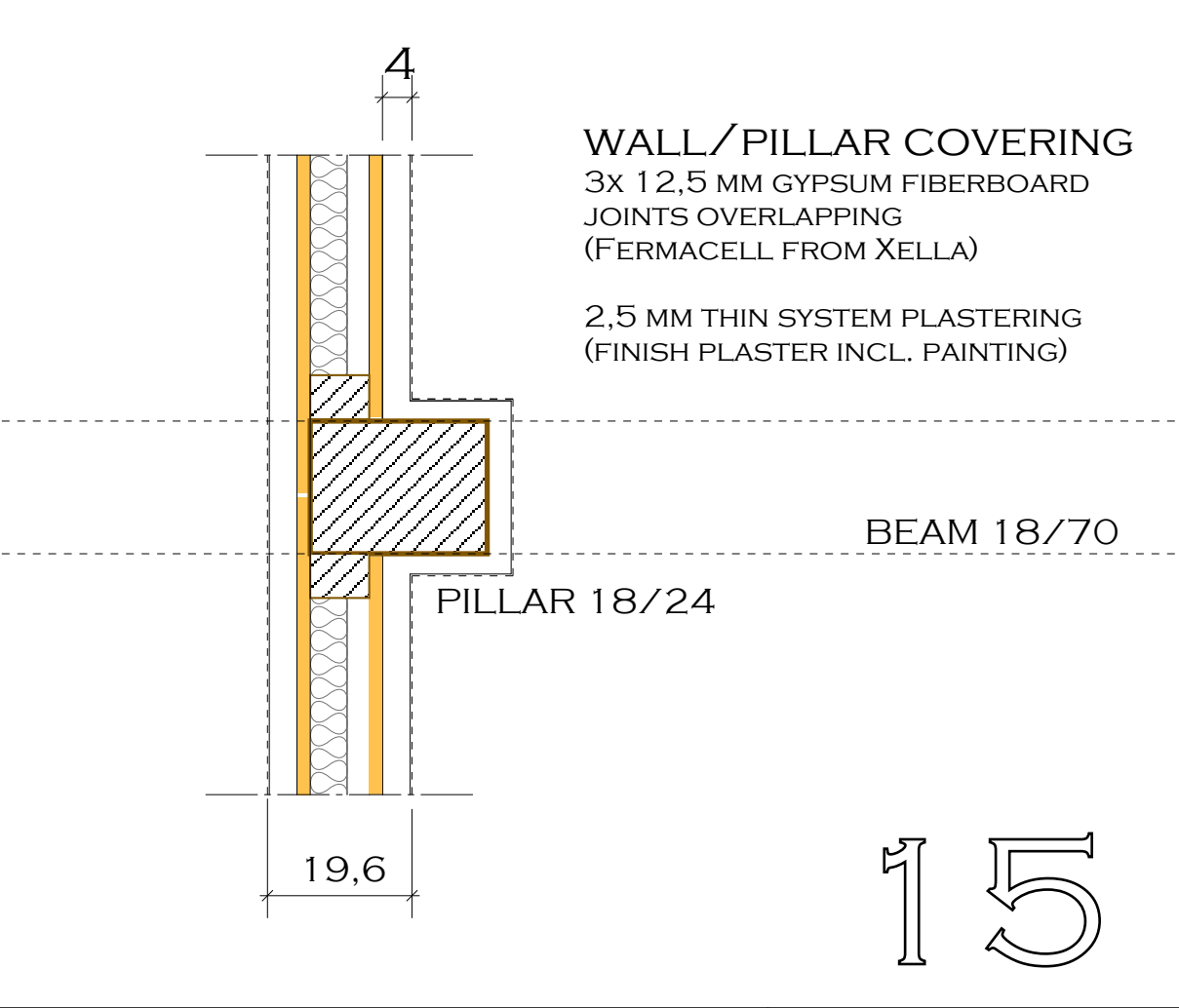
PILLAR DECOR IN PERIM. WALL 1/10



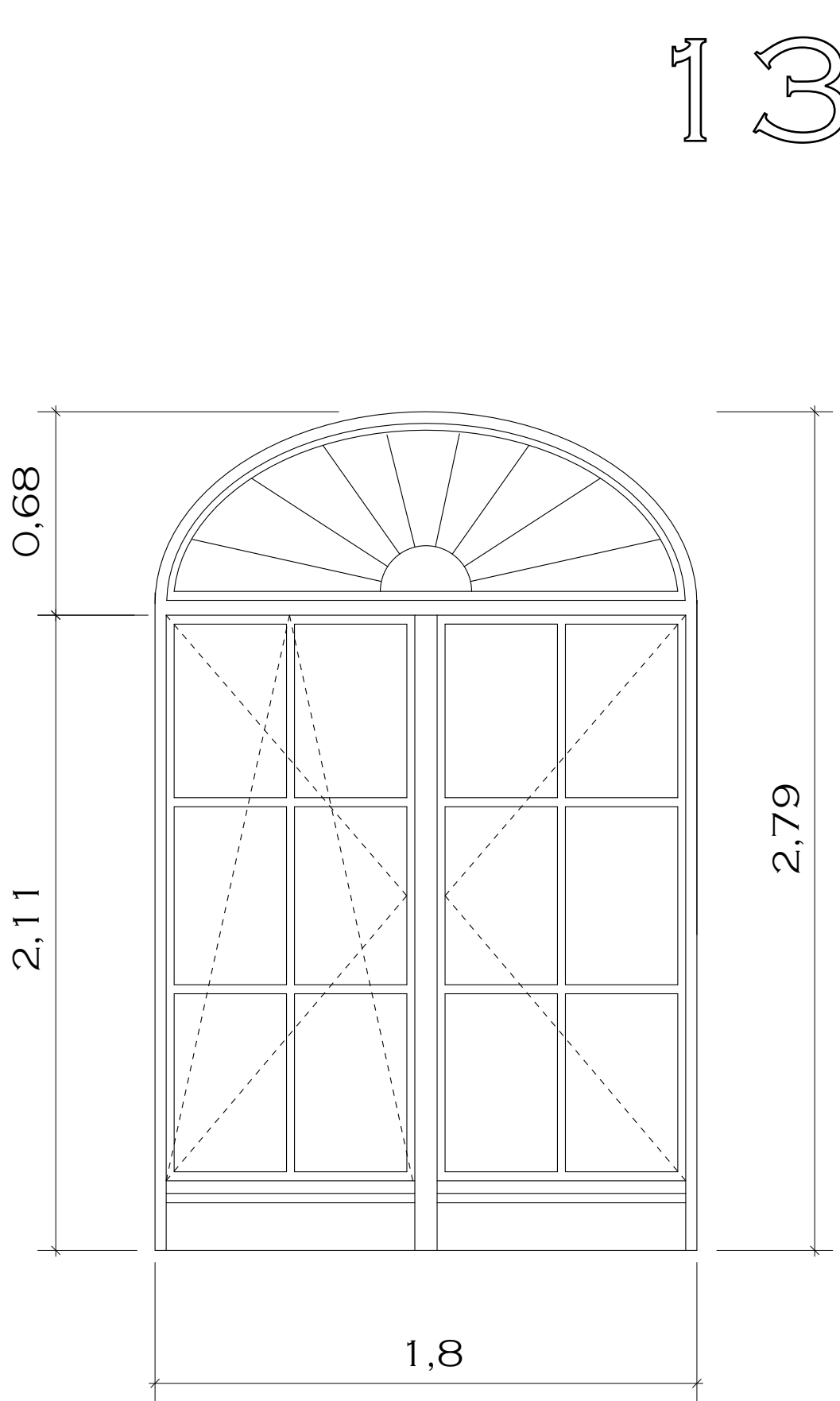
FAÇADE DECORATION 1/20



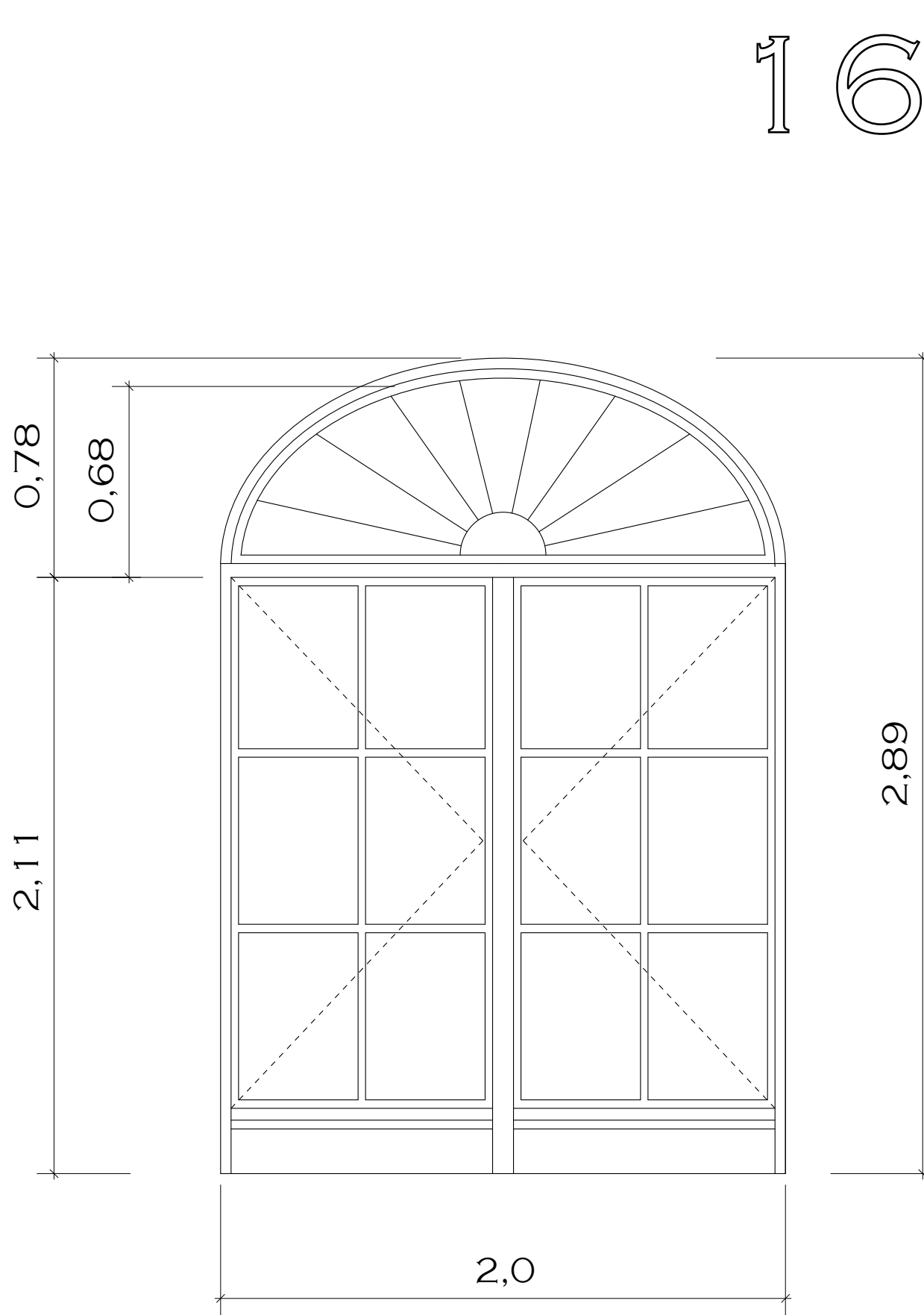
PILLAR POSITION IN INNER WALL 1/10



WINDOW 1/20



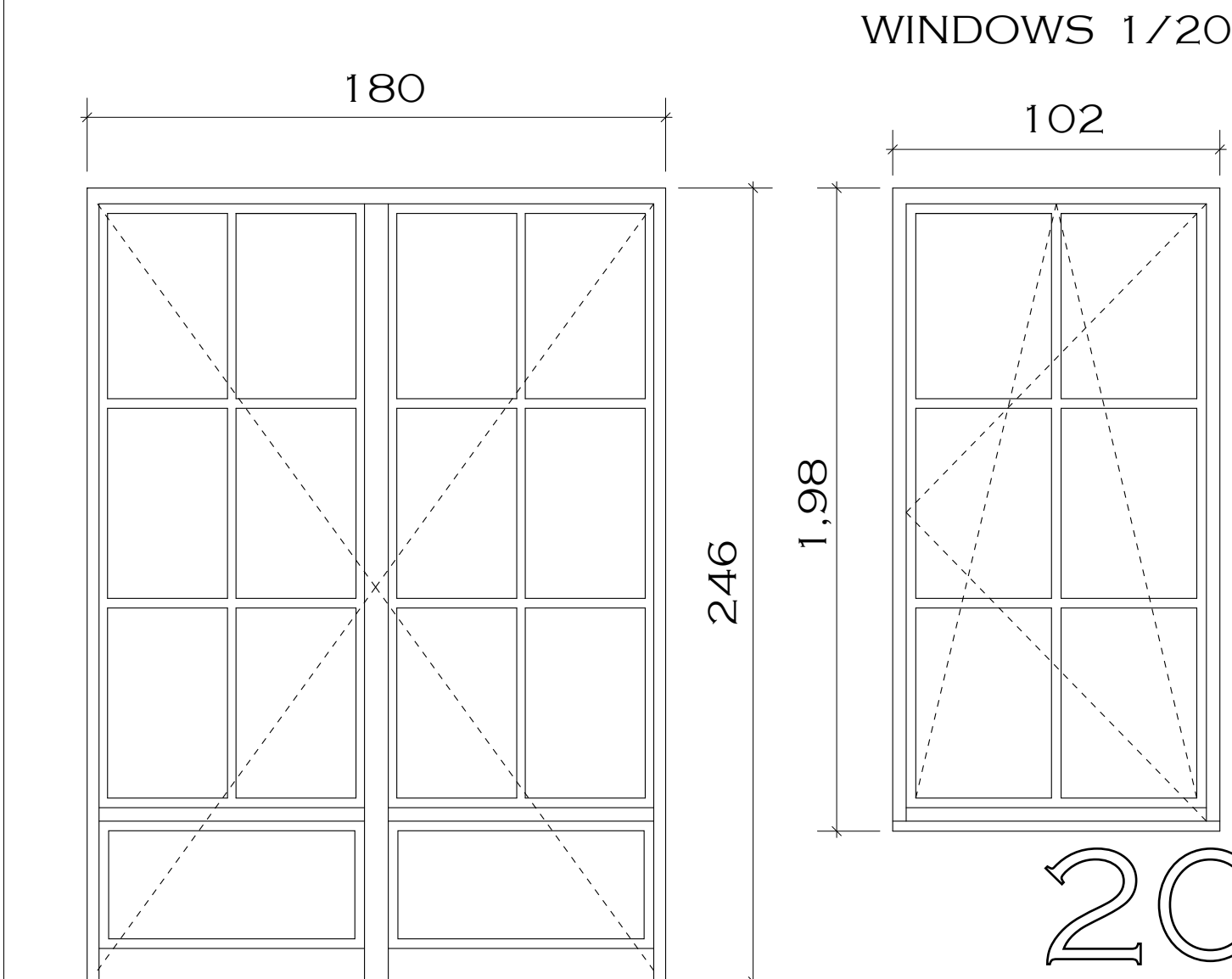
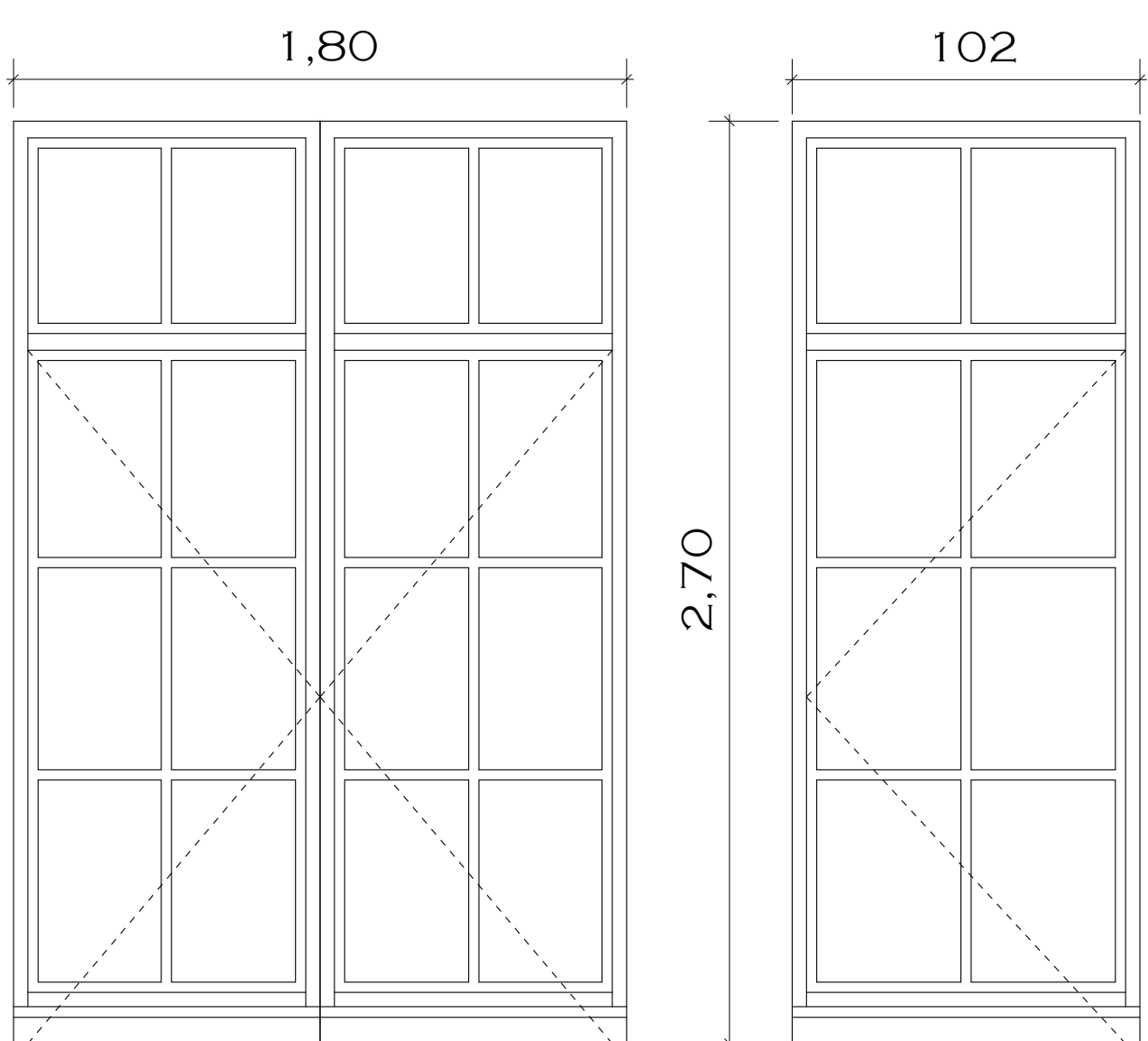
ENTRANCE DOOR 1/20



ALL WINDOW
DIMENSIONS
TO VERIFY
BY WORKING
DRAWINGS

WINDOWS
1/20

18



WINDOWS 1/20

20



**MAHARISHI
INVINCIBILITY
SCHOOLS**

LOCATION

CLIENT
ONTWIKKELINGSMAATSCHAPPIJ
FORTUINLIJK WONEN BV. (OFW)
DONAUSTRAAT 176
8226 LC LELYSTAD
NEDERLAND

PROJECT MANAGER
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CRISTIAN SCHWEIZER
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TEL. 0031475 538541

PROJECT ARCHITECT
ALBERTO CASTAÑO
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23911 MUSTIN
TEL. 0049 4546 891 170
TEL. 0031475 538541

BUILDING COMPANY

TEL.

BUILDING SITE MANAGER

TEL.

STRUCTURAL ENGINEERING

TEL.

OFFICIAL SURVEYOR

TEL.

HOUSE TECHN. ENGINEERING

TEL.

LEGENDA

FFL-GF	0,00 M
-FINISH FLOOR LEVEL GROUND FLOOR- FFL-GF	+3,48 M
-FINISH FLOOR LEVEL FIRST FLOOR- TERRAIN	-0,48 M
-FINISH FLOOR LEVEL SOIL- FFL-W	+4,38 M
-FINISH FLOOR LEVEL WINDOW- FLAT ROOF SLOPE CA.1°	CA.1°

MODULE

OUTER WALL THICKNES	0,30 M
CLASS ROOM WALL TH.	0,25 M
CEILING HIGHT	0,42 M

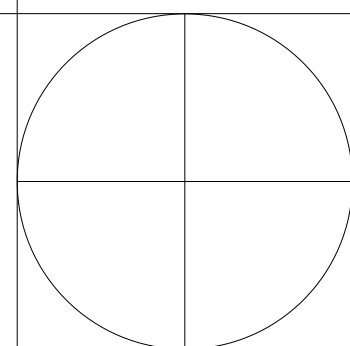
ALL DIMENSIONS TO BE
VERIFIED BY THE STRUCT.
CALCULATIONS

PRELIMINARY PROJECT
BASIC DETAILS 3

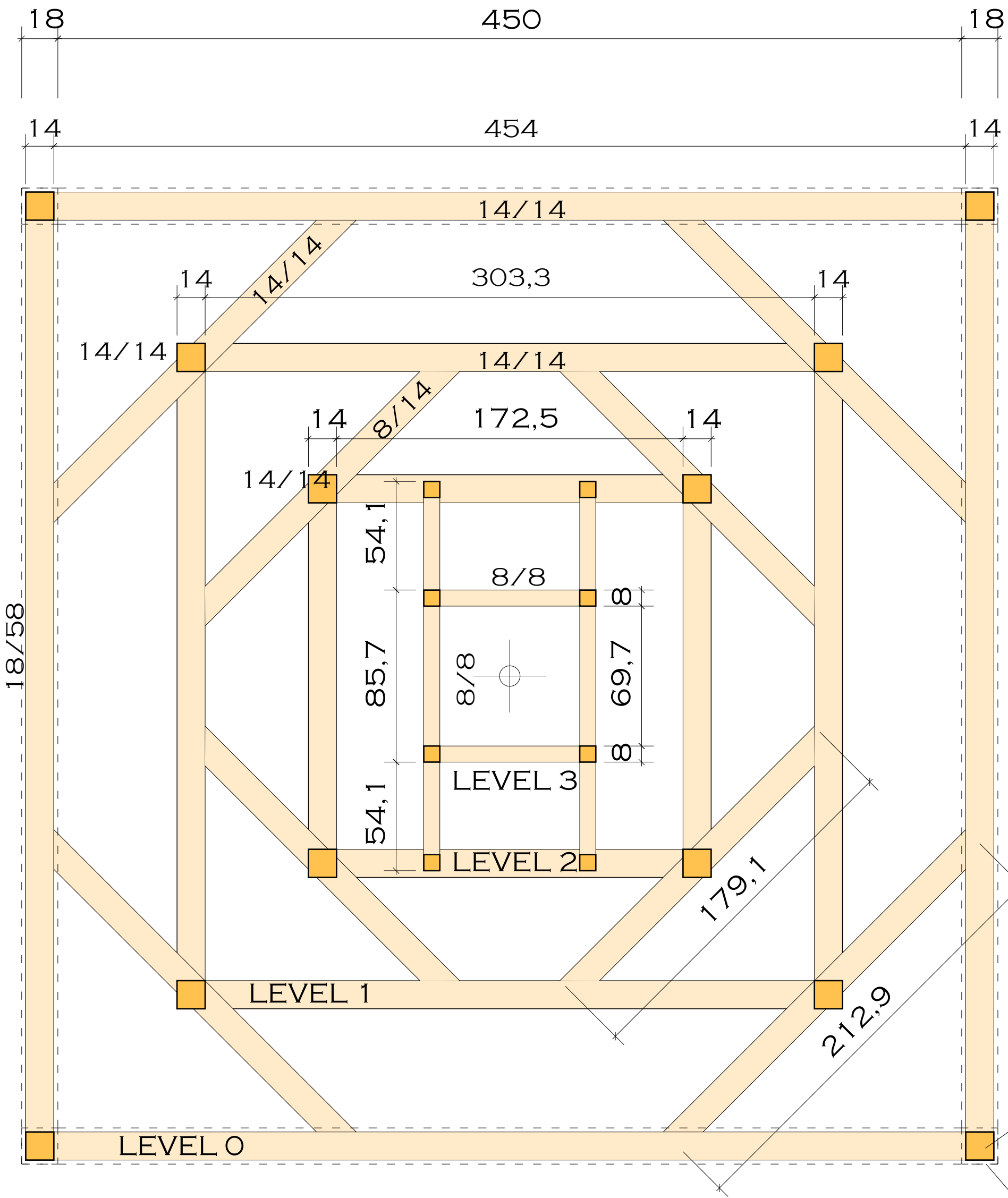
SCALE 1/20	DESIGN CA.
DATE 31.05.2007	CHANGE

DRWG. NR.

BD3



FLOOR PLAN 1/20



TOP-LEVEL ORNAMENT +11,34

BOTTOM-LEVEL ORNAMENT +10,38

TOP-LEVEL- OUTER WALL +7,98

14/14
UPER PILLAR

18/18
LOWER PILLAR

LEGENDA DOME

ROOF

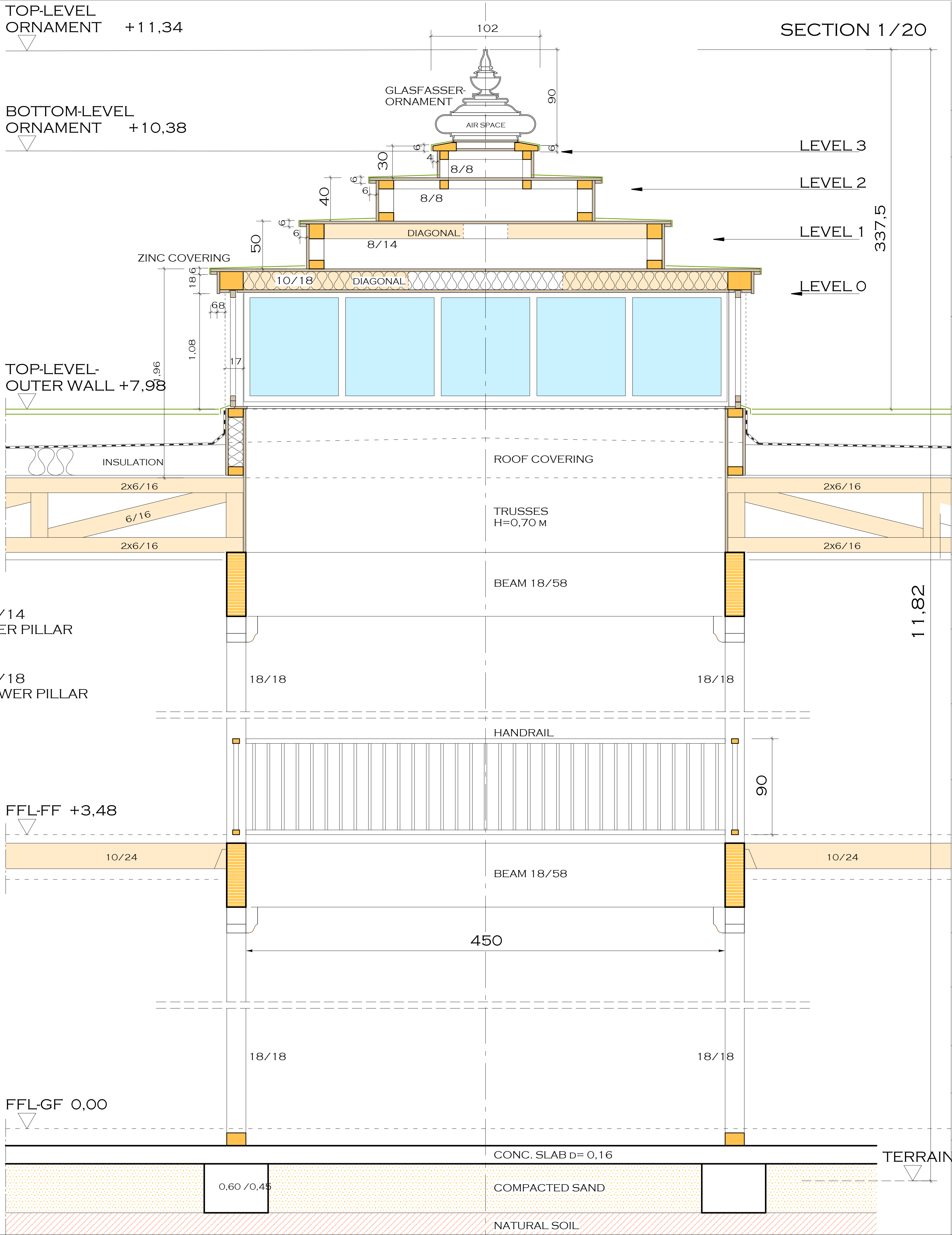
ZINC COVERING SHEET
22 MM OSB BOARD
WOOD CONSTRUCTION
10 CM INSULATION ON LEVEL 0
22 MM OSB BOARD

DOUBLE GLAZED WINDOWS

WALLS

WOOD CONSTRUCTION
WATERPROOFED WOODEN BOARDS
(I.E. WERZALITH OR SIML.)

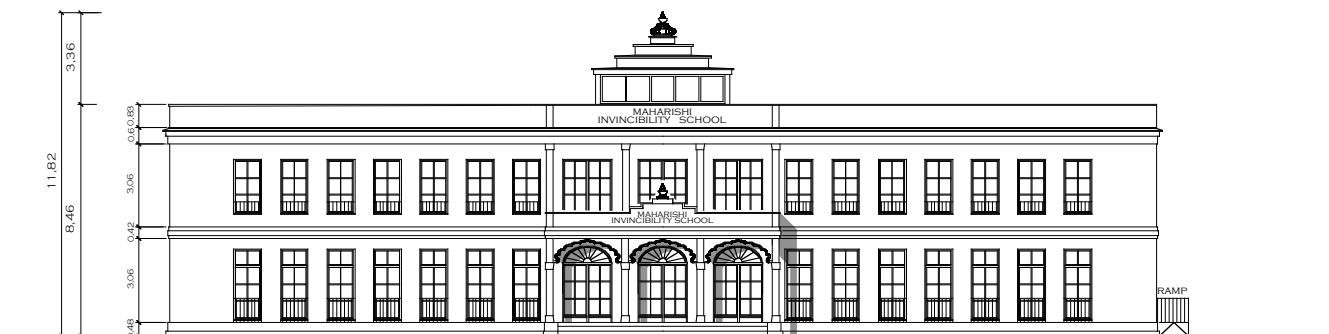
SECTION 1/20



 MAHARISHI INVINCIBILITY SCHOOLS	
LOCATION	
CLIENT ONTWIKKELINGSMAATSCHAPPIJ FORTUINLIJK WONEN BV. (OFW) DONAUSTRAAT 176 8226 LC LELYSTAD NEDERLAND	
PROJECT MANAGER FREIER ARCHITEKT+STADTPLANER CRISTIAN SCHWEIZER STATION 24 6063 NP VLODROP NEDERLAND TEL. 0031475 538541	
PROJECT ARCHITECT ALBERTO CASTAÑO FREISCHAFFENDER ARCHITEKT WALDSTRASSE 20 C 23911 MUSTIN TEL. 0049 4546 891 170 TEL. 0031475 538541	
BUILDING COMPANY	
TEL.	
BUILDING SITE MANAGER	
TEL.	
STRUCTURAL ENGINEERING	
TEL.	
OFFICIAL SURVEYOR	
TEL.	
HOUSE TECHN. ENGINEERING	
TEL.	
LEGENDA FFL-GF 0,00 M -FINISH FLOOR LEVEL GROUND FLOOR- FFL-GF +3,48 M -FINISH FLOOR LEVEL FIRST FLOOR- TERRAIN -0,48 M -FINISH FLOOR LEVEL SOIL- FFL-W +4,38 M -FINISH FLOOR LEVEL WINDOW- FLAT ROOF SLOPE CA.1° MODULE M OUTER WALL THICKNES 0,30 M CLASS ROOM WALL TH. 0,25 M CEILING HIGHT 0,42 M	
ALL DIMENSIONS TO BE VERIFIED BY THE STRUCT. CALCULATIONS	
PRELIMINARY PROJECT BASIC DETAILS 1	
SCALE 1/20	DESIGN CA.
DATE 31.05.2007	CHANGE
DRWG. NR.	
BD 1	

Statische Berechnung

!Vorstatik zur Ermittlung Maßgebender Querschnitte!



Bauherr : MFI International, Holland

Rechtsträger : MFI International, Holland, Station 24, 6063 NP Vlodrop, The Netherlands

Bauort : noch nicht bekannt / Annahme Deutschland Schneelastzone III > 400m über NN

Bauvorhaben : Seminargebäude / Schule

Architekt : Dipl.-Ing. Alberto Castaño, Waldstr. 20c, D-23911 Mustin

Bearbeitet : Dipl.-Ing. (FH) Sven Schütrumpf

Inhaltsverzeichnis

Berechnungsgrundlagen	3
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Grundriss Erdgeschoß	7
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Pos 2 Fachwerkbinder	10
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Pos 30 Streifenfundament	22
Pos 30.1 Streifenfundament	24

Berechnungsgrundlagen

DIN 1055 Lastannahmen für Bauten

DIN V ENV 1991 Grundlagen der Tragwerksplanung und Einwirkungen auf Tragwerke

DIN 1045 Tragwerke aus Beton, Stahlbeton und Spannbeton

DIN 1052 Holzbauwerke, Berechnung und Ausführung

DIN 1053 Berechnungsgrundlagen für Bauteile aus künstlichen und natürlichen Steinen

DIN 1054 Zulässige Belastung des Baugrundes

Literatur: Schneider Bautabellen für Ingenieure; Wendehorst Bautechnische Zahlentafeln

Grundlegende Baustoffe

Mindestanforderungen:

Beton:	C 16/20 X0
Stahlbeton	C 20/25 XC2 Gründungsbauteile
Betonstahl:	BSt 500 S + M (A)
Profilstahl:	S 235
Mauerwerk:	siehe statische Berechnungen
Holz:	NH Sortierklasse S 10 / BSH 11

Baugrund

Eine Geotechnische Untersuchung bzw. ein Geotechnischer Bericht zur Baugrunduntersuchung nach DIN 4020 zur Feststellung:

- Zum Baugrundaufbau mit Schichten, Verwerfungen, Störungen und Einschlüssen;
- Zu den Grundwasserverhältnissen;
- Zu den boden- und felsmechanischen Eigenschaften und Kenngrößen;
- Zu den Rändern der untersuchten Baugrundbereiche und Randbedingungen an diesen;

!Ist nicht vorhanden!

Annahmen siehe statische Berechnungen.

Baubeschreibung

Angaben zum Standort:

- Umgebungssituation: noch nicht bekannt
- NN – Höhenlage: noch nicht bekannt
- Schneelastzone: noch nicht bekannt
- Geografische Sonderlage: noch nicht bekannt

Angaben zur Nutzung des Bauwerks

- Nutzung: Seminar- und Tagungsgebäude
- Gebäudeklasse: **je nach Landesbauordnung**
- Brandschutz: **je nach Landesbauordnung bzw. Gebäudeklasse**

(Aufgrund der Nutzung wird es besondere Anforderungen an den Brandschutz geben, beachte Landesvorschriften.)

Abmessungen des Gebäudes

- Länge: 36,06m
- Breite: 22,86m
- Höhe: 11,82m ü. OKG
- Abtreppungen: nicht vorhanden

Erläuterung der statischen Grundkonzeption

- Grundprinzip: Holzständerbauweise
- Fundament: Streifenfundament
- Decken: Holzbalkendecken
- Dach: Fachwerkbinder
- Wände: Holzständerkonstruktion

Einwirkungen

Dach:

- Verkehrslasten: 2,0 kN/m², der Windsog ist zu berücksichtigen

Decken:

- Verkehrslasten: 3,5 kN/m², „Klassenzimmer“ bzw. Seminarräumen
- Verkehrslasten: 5,0 kN/m² Flure zu „Klassenzimmern“ bzw. Seminarräumen

Treppen:

- Verkehrslasten: 5,0 kN/m², kein Wohngebäude

Balkone:

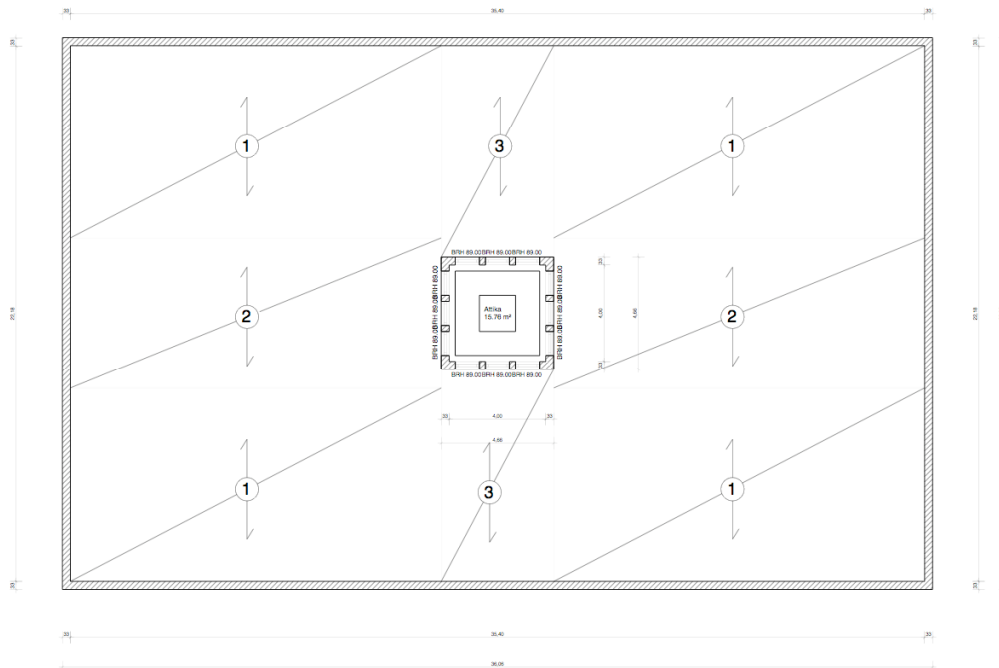
- Verkehrslasten: $\Rightarrow 10\text{m}^2$ 3,5 kN/m²; $\Rightarrow < 10\text{m}^2$ 5,0 kN/m²

Trennwandzuschlag für Decken:

- 0,75kN/m² (g = 100 kg/m²)
- 1,25kN/m² (g= 150 kg/m²)

!!Vorbemessung!!

Grundriss Dachgeschoß

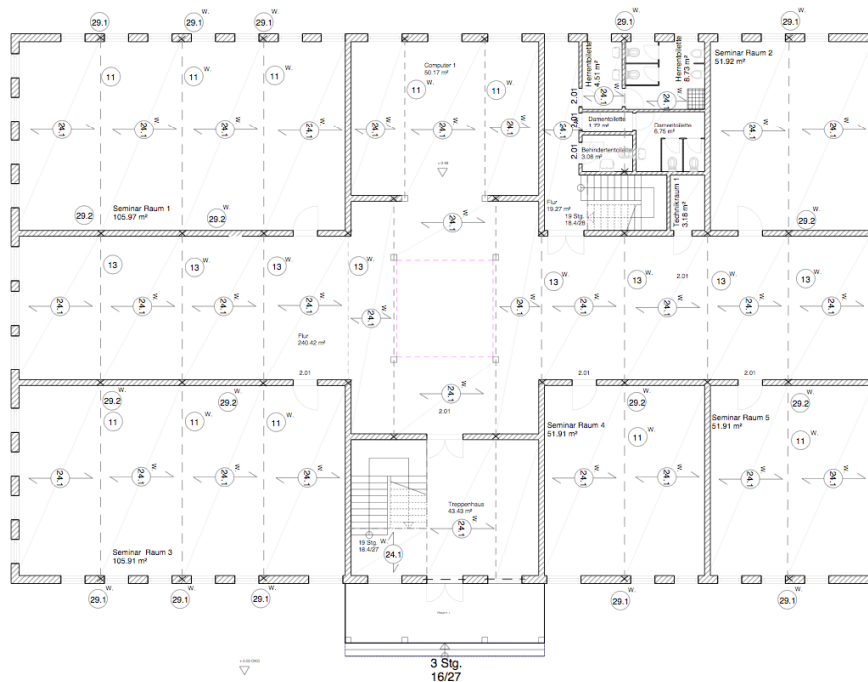


Positionsplan, Fachwerkträger

Projekt:	Seminarzentrum
Bauherr:	Maharishi Weltfriedens-Stiftung Flechtträger: Maharishi Veda GmbH
Architekt:	Herr Roberto Cattaneo Bearbeiter: Dipl.-Ing. (FH) Sven Schürumpf
Datename:	Invisibility School
Mailstat:	1 : 100 Datum: 26.04.2007 / 1. Dachgeschoß

!!Vorbemessung!!

Grundriss Erdgeschoß



Vorabzug Positionsplan

Projekt: Seminarzentrum		
Bauherr: Maharishi Weltfriedens-Stiftung Fremdträger: Maharishi Veda GmbH		
Architekt: Bearbeiter: Dipl. Ing. (FH) Sven Schürumpf		
Dateiname: Invisibility School		
Maßstab:	1 : 100 Datum 22.05.2007	Erdgeschoß

Dachgeschoß

Lastannahmen

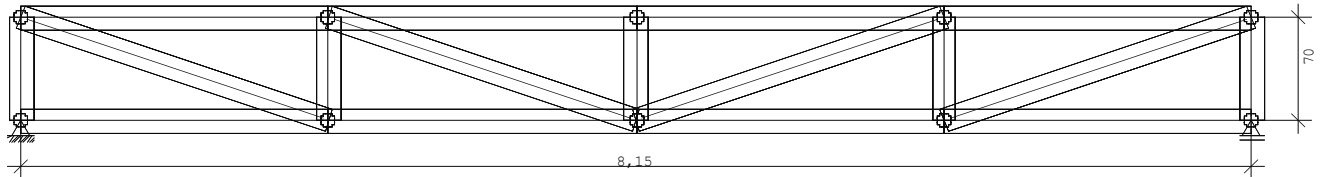
	kN/m ²
Dachabdichtungsbahn 1,54kg/m ²	0,02
EPS 040 DAA d =18cm – d= 28cm	0,24
Trennlage aus PE-Folie, 0,2mm dick	0,02
Holzschalung (Nut und Feder) 28mm	0,23
Fachwerkträger a = 70cm	0,23
Dampfsperre	0,01
Gipskartonplatte d=2,5cm	0,28
Ständige Lasten g	1,12
Verkehrslasten q	2,00

Pos 1 Fachwerkbinder

Berechnungsgrundlagen: DIN 1052 , DIN 1055

Baustoffe : Nadelholz S 10 Gamma = 6.00 kN/m³
 Stahl St 37, Holzschutz nach DIN 68800

Maßstab 1 : 50



SYSTEM: Parallelgurtiger Fachwerkbinder Art 1 mit 4 Feldern

Spannweite L = 8.15 m
 Feldlänge L1 = 2.04 m
 Binderhöhe H = 0.70 m
 Binderabstand a = 1.00 m
 Queraussteifung OG sB = 1.00 m

BELASTUNG und Lastfälle:	[kN/m ²]	[kN/m]		
Auflast OG	go = 0.55 Dfl	0.55		
Unterkonstruktion UG	gu = 0.40 Gfl	0.40		
Verkehr auf OG	p = 1.00 Dfl	1.00		
Schnee	s0 = 1.00 Gfl	s = 1.00	ks = 1.00	
Wind	q = 0.50	wd = 0.00	ws = 0.30 kN/m	

Bindergewicht G = 1.87 kN 0.23 kN/m

LF H : Eigengewicht + go + gu + p + s

LF HZ: Eigengewicht + go + gu + p + s + wd + ws

!!Vorbemessung!!

AUFLAGERKRÄFTE:	V li	H li	V re	H re [kN]
g	4.81	0.00	4.81	0.00
Ständig OG	2.24	0.00	2.24	0.00
Schnee voll	4.07	0.00	4.07	0.00
LF H	12.96	0.00	12.96	0.00
LF HZ	12.96	0.00	12.96	0.00

DURCHBIEGUNG : zul f = 1.36 cm = L/600 Kriechfaktor = 1.00
 Untergurt max f = 0.94 cm < L/600

Überhöhung des Binders nach DIN 1052 8.5.5

STABKRÄFTE: LF H [kN]				
Nr.	Untergurt	Obergurt	Vertikalstäbe	Diagonalen
1	0.00	-28.24	-12.48	29.84
2	28.24	-37.66	-8.68	9.93
3			-5.38	

SPANNUNGSNACHWEISE

Obergurt 7,5 / 16 l = 2.04 m A = 120 cm²

N = -37.66 kN MF = 1.36 kNm Omega = 1.35 kB = 1.00
 Sigma / zul Sigma = 3.14 / 8.50 + 4.25 / 10.00 = 0.79 < 1.0
 Sigma / zul Sigma = 3.14 / 6.30 + 4.25 / 11.00 = 0.88 < 1.0

Untergurt 7,5 / 16 l = 2.04 m A = 120 cm² Delta A = 15 %

N = 28.24 kN MF = 0.24 kNm
 Sigma / zul Sigma = 2.35 / 7.00 + 0.77 / 10.00 = 0.41 < 1.0
 N = 28.24 kN MS = 0.00 kNm (Nettowerte)
 Sigma / zul Sigma = 2.77 / 7.00 = 0.41 < 1.0

Pfosten 6 / 16 l = 0.70 m A = 96 cm²

N = -12.48 kN Omega = 1.27 Lambda = 40.41
 Sigma / zul Sigma = 1.30 / 6.70 = 0.19 < 1.0

Diagonale 6 / 16 l = 2.15 m A = 96 cm² Delta A = 15 %

N = 29.84 kN (Nettowerte)
 Sigma / zul Sigma = 3.66 / 7.00 = 0.52 < 1.0

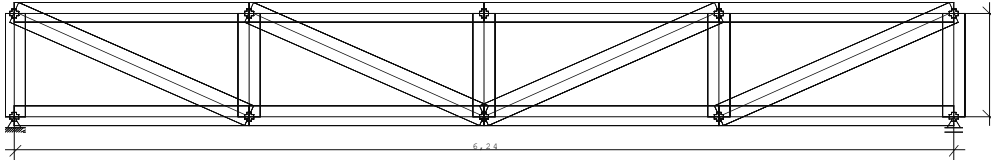
!!Vorbemessung!!

Pos 2 Fachwerkbinder

Berechnungsgrundlagen: DIN 1052 , DIN 1055

Baustoffe : Nadelholz S 10 $\gamma = 6.00 \text{ kN/m}^3$
Stahl St 37, Holzschutz nach DIN 68800

Maßstab 1 : 50



SYSTEM: Parallelgurtiger Fachwerkbinder Art 1 mit 4 Feldern

Spannweite $L = 6.24 \text{ m}$
Feldlänge $L1 = 1.56 \text{ m}$
Binderhöhe $H = 0.70 \text{ m}$
Binderabstand $a = 1.00 \text{ m}$
Queraussteifung OG $sB = 1.00 \text{ m}$

BELASTUNG und Lastfälle: [kN/m²] [kN/m]

Auflast OG	$g_o = 0.55$	Dfl	0.55		
Unterkonstruktion UG	$g_u = 0.40$	Gfl	0.40		
Verkehr auf OG	$p = 1.00$	Dfl	1.00		
Schnee	$s_0 = 1.00$	Gfl	$s = 1.00$	$k_s = 1.00$	
Wind	$q = 0.50$		$w_d = 0.00$	$w_s = 0.30 \text{ kN/m}$	

Bindergewicht $G = 1.31 \text{ kN}$ 0.21 kN/m

LF H : Eigengewicht + $g_o + g_u + p + s$

LF HZ: Eigengewicht + $g_o + g_u + p + s + w_d + w_s$

AUFLAGERKRÄFTE: V_{li} H_{li} V_{re} H_{re} [kN]

g	3.62	0.00	3.62	0.00
Ständig OG	1.72	0.00	1.72	0.00
Schnee voll	3.12	0.00	3.12	0.00
LF H	9.86	0.00	9.86	0.00
LF HZ	9.86	0.00	9.86	0.00

DURCHBIEGUNG : zul $f = 1.04 \text{ cm} = L/600$ Kriechfaktor = 1.00

Untergurt max $f = 0.39 \text{ cm} < L/600$

!!Vorbemessung!!

Überhöhung des Binders nach DIN 1052 8.5.5

STABKRÄFTE: LF H [kN]

Nr.	Untergurt	Obergurt	Vertikalstäbe	Diagonalen
1	0.00	-16.45	-9.50	18.01
2	16.45	-21.93	-6.62	5.99
3			-4.11	

SPANNUNGSNACHWEISE

Obergurt 6 / 16 l = 1.56 m A = 96 cm²

N = -21.93 kN MF = 0.79 kNm Omega = 1.57 kB = 1.00
Sigma / zul Sigma = 2.28 / 8.50 + 3.10 / 10.00 = **0.58 < 1.0**
Sigma / zul Sigma = 2.28 / 5.42 + 3.10 / 11.00 = **0.70 < 1.0**

Untergurt 6 / 16 l = 1.56 m A = 96 cm² Delta A = 15 %

N = 16.45 kN MF = 0.14 kNm
Sigma / zul Sigma = 1.71 / 7.00 + 0.54 / 10.00 = **0.30 < 1.0**
N = 16.45 kN MS = 0.00 kNm (Nettowerte)
Sigma / zul Sigma = 2.02 / 7.00 = **0.30 < 1.0**

Pfosten 6 / 16 l = 0.70 m A = 96 cm²

N = -9.50 kN Omega = 1.27 Lambda = 40.41
Sigma / zul Sigma = 0.99 / 6.70 = **0.15 < 1.0**

Diagonale 6 / 16 l = 1.71 m A = 96 cm² Delta A = 15 %

N = 18.01 kN (Nettowerte)
Sigma / zul Sigma = 2.21 / 7.00 = 0.32 < 1.0

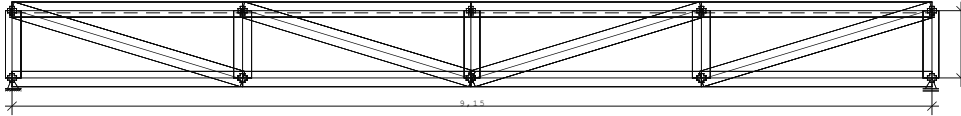
!!Vorbemessung!!

Pos 3 Fachwerkbinder

Berechnungsgrundlagen: DIN 1052 , DIN 1055

Baustoffe : Nadelholz S 10 $\gamma = 6.00 \text{ kN/m}^3$
Stahl St 37, Holzschutz nach DIN 68800

Maßstab 1 : 75



SYSTEM: Parallelgurtiger Fachwerkbinder Art 1 mit 4 Feldern

Spannweite $L = 9.15 \text{ m}$
Feldlänge $L1 = 2.29 \text{ m}$
Binderhöhe $H = 0.70 \text{ m}$
Binderabstand $a = 1.00 \text{ m}$
Queraussteifung OG $sB = 1.00 \text{ m}$

BELASTUNG und Lastfälle: [kN/m²] [kN/m]

Auflast OG	$g_o = 0.55$	Dfl	0.55		
Unterkonstruktion UG	$g_u = 0.40$	Gfl	0.40		
Verkehr auf OG	$p = 1.00$	Dfl	1.00		
Schnee	$s_0 = 1.00$	Gfl	$s = 1.00$	$k_s = 1.00$	
Wind	$q = 0.50$		$wd = 0.00$	$ws = 0.30 \text{ kN/m}$	

Bindergewicht $G = 2.07 \text{ kN}$ 0.23 kN/m

LF H : Eigengewicht + $g_o + g_u + p + s$
LF HZ: Eigengewicht + $g_o + g_u + p + s + wd + ws$

AUFLAGERKRÄFTE: V li H li V re H re [kN]

g	5.38	0.00	5.38	0.00
Ständig OG	2.52	0.00	2.52	0.00
Schnee voll	4.57	0.00	4.57	0.00
LF H	14.53	0.00	14.53	0.00
LF HZ	14.53	0.00	14.53	0.00

DURCHBIEGUNG : zul $f = 1.53 \text{ cm} = L/600$ Kriechfaktor = 1.00

Untergurt max $f = 1.51 \text{ cm} < L/600$

Überhöhung des Binders nach DIN 1052 8.5.5

STABKRÄFTE: LF H [kN]

Nr.	Untergurt	Obergurt	Vertikalstäbe	Diagonalen
1	0.00	-37.07	-13.99	38.75
2	37.07	-48.27	-10.20	11.69
3			-5.63	

!!Vorbemessung!!

SPANNUNGSNACHWEISE

Obergurt **8 / 16** $l = 2.29 \text{ m}$ $A = 120 \text{ cm}^2$

N = -48.27 kN MF = -1.06 kNm Omega = 1.41 kB = 1.00
Sigma / zul Sigma = 4.02 / 8.50 + 3.30 / 10.00 = 0.80 < 1.0
Sigma / zul Sigma = 4.02 / 6.05 + 3.30 / 11.00 = **0.97 < 1.0**

Untergurt **8 / 16** $l = 2.29 \text{ m}$ $A = 120 \text{ cm}^2$ Delta A = 15 %

N = 37.07 kN MF = 0.31 kNm
Sigma / zul Sigma = 3.09 / 7.00 + 0.96 / 10.00 = 0.54 < 1.0
N = 37.07 kN MS = 0.00 kNm (Nettowerte)
Sigma / zul Sigma = 3.63 / 7.00 = 0.54 < 1.0

Pfosten **6 / 16** $l = 0.70 \text{ m}$ $A = 96 \text{ cm}^2$

N = -13.99 kN Omega = 1.27 Lambda = 40.41
Sigma / zul Sigma = 1.46 / 6.70 = 0.22 < 1.0

Diagonale **6 / 16** $l = 2.39 \text{ m}$ $A = 96 \text{ cm}^2$ Delta A = 15 %

N = 38.75 kN (Nettowerte)
Sigma / zul Sigma = 4.75 / 7.00 = 0.68 < 1.0

Pos 24.1 Deckenbalken

(Seminarraum)

Decke Spannweite 3,54m

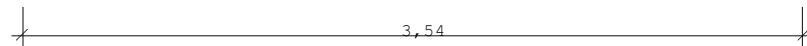
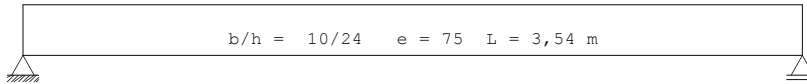
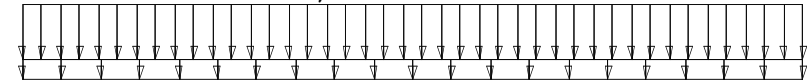
	kN/m²
Fliesen, d=1,0cm	0,22
Fermacell Estrich-Elemente d= 2,5cm	0,30
Trittschalldämmung, d = 2cm	0,02
OSB - Platte d= 2,2cm	0,24
Deckenbalken 10/24cm a = 0,45cm	
mineralischer Faserdämmstoff d= 10cm	0,10
Lattung d/b 3/5cm	0,02
Federschienen	0,01
Fermacell Platten - Elemente d = 2,5cm	0,30
g	1,21

Verkehrslast

q 3,50

BAUSTOFF: Nadelholz S 10
Holzfeuchte $\leq 18 \%$ $k_e = 1.000$ $k_s = 1.000$

Maßstab 1 : 34,2



BELASTUNG Flächenlast $g_1 = 1.25 \text{ kN/m}^2$ $p_1 = 3.50 \text{ kN/m}^2$

BALKEN: 10 / 24 $e = 0.75 \text{ m}$ LF H

max M_f =	5.58 kNm	Eta_Sigma =	5.81 / 10.00 = 0.58 < 1.0
max M_s =	0.00 kNm	Eta_Sigma =	0.00 / 10.00 = 0.00 < 1.0
max Q =	-6.31 kN	Eta_Tau =	0.39 / 0.90 = 0.44 < 1.0
max f =	0.63 cm < $L/300$	=	1.18 Eta_f = 0.54 < 1.0

AUFLAGERKRÄFTE (kN/m)

Stütze	aus g	aus p	Vollast	max	min
1	2.21	6.20	8.41	8.41	2.21
2	2.21	6.20	8.41	8.41	2.21
Summe:	4.43	12.39	16.82	16.82	4.43

Unterzüge

Pos 11 Unterzug

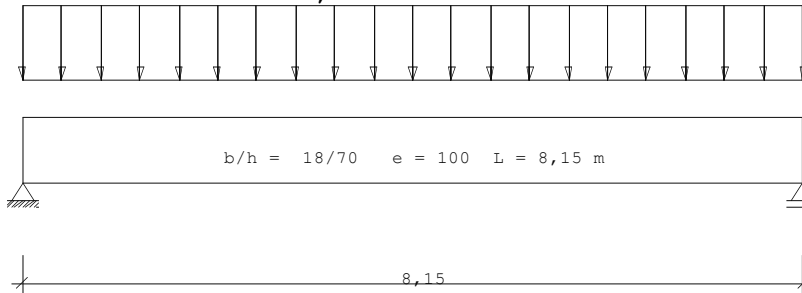
Belastung aus Pos. 24.1 und W. 24.1

8,45+8,45= ~ 16,90 kN/m + Eigenlast

BAUSTOFF: Brettschichtholz BS 11

Holzfeuchte $\leq 18 \%$ $k_e = 1.000$ $k_s = 1.000$

Maßstab 1 : 78,8



BELASTUNG Flächenlast $g_1 = 17.55 \text{ kN/m}^2$ $p_1 = 0.00 \text{ kN/m}^2$

BALKEN: 18 / 70 $e = 1.00 \text{ m}$ LF H

max $M_f = 145.71 \text{ kNm}$	$\text{Eta_Sigma} = 9.91 / 11.00 = 0.90 < 1.0$
max $M_s = 0.00 \text{ kNm}$	$\text{Eta_Sigma} = 0.00 / 11.00 = 0.00 < 1.0$
max $Q = 71.52 \text{ kN}$	$\text{Eta_Tau} = 0.85 / 1.20 = 0.71 < 1.0$
max $f = 1.78 \text{ cm} < L/300$	$= 2.72 \text{ Eta_f} = 0.66 < 1.0$

AUFLAGERKRÄFTE (kN/m)

Stütze	aus g	aus p	Vollast	max	min
1	71.52	0.00	71.52	71.52	71.52
2	71.52	0.00	71.52	71.52	71.52
Summe:	143.03	0.00	143.03	143.03	143.03

!!Vorbemessung!!

Pos 13 Unterzug

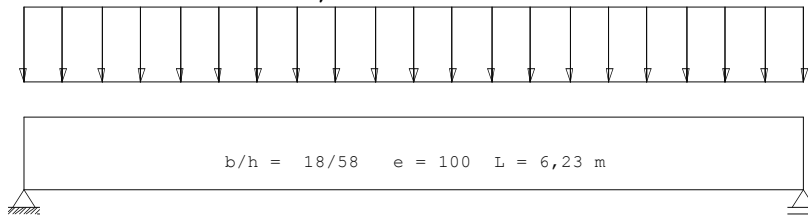
Belastung aus Pos. 24.1.1 und W. 24.1.1

11+11= ~ 22,00 kN/m + Eigenlast

BAUSTOFF: Brettschichtholz BS 11

Holzfeuchte $\leq 18 \%$ $k_e = 1.000$ $k_s = 1.000$

Maßstab 1 : 60,2



6,23

BELASTUNG Flächenlast $g_1 = 22.00$ kN/m² $p_1 = 0.00$ kN/m²

BALKEN: 18 / 58 e = 1.00 m LF H

max $M_f = 106.74$ kNm $\text{Eta_Sigma} = 10.58 / 11.00 = 0.96 < 1.0$
max $M_s = 0.00$ kNm $\text{Eta_Sigma} = 0.00 / 11.00 = 0.00 < 1.0$
max $Q = 68.53$ kN $\text{Eta_Tau} = 0.98 / 1.20 = 0.82 < 1.0$
max $f = 1.34$ cm $< L/300 = 2.08$ $\text{Eta_f} = 0.65 < 1.0$

AUFLAGERKRÄFTE (kN/m)

Stütze aus g aus p Vollast max min
1 68.53 0.00 68.53 68.53 68.53
2 68.53 0.00 68.53 68.53 68.53
Summe: 137.06 0.00 137.06 137.06 137.06

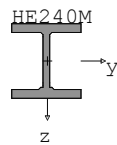
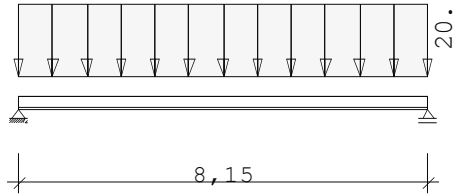
Alternativ zu Pos 11 und Pos 13

(Pos 11)

Bemerkung: Deckenbalken Pos 24.1 können als Zweifeldträger ausgebildet werden. Auflager beachten!

EINFELDTRÄGER HE 240 M L = 8.15 m S235

Maßstab 1 : 150



Teilsicherheitsbeiwert $\Gamma_{M1} = 1.10$ Eigengewicht $g = 1.570 \text{ kN/m}$

EINFACHE LASTEN (q-Anteile : $\Gamma_{F1} = 1.50$; z_p : Oberseite)

Streckenlast 1 : $g_z = 20.60 \text{ kN/m}^2$ $b_z = 1.00 \text{ m}$

AUFLAGERKRÄFTE Grundkomb. (ohne Γ_{F1}) * = Maximum

Lager	Ew	V (kN)	Hx (kN)	Hy (kN)	aus Lasten **)
links	G	90.34	0.00	0.00	
rechts	G	90.34	0.00	0.00	

SPANNUNGEN Grundkomb. nach Th.1.O. , $x_0 = 4.08 \text{ m}$ Γ_{F1} -fach

vorh. σ_x / f_{yd}	=	138.11 / 218.2	=	0.63	< 1
vorh. τ / τ_{Rd}	=	0.00 / 126.0	=	0.00	< 1
vorh. σ_v / f_{yd}	=	138.11 / 218.2	=	0.63	< 1

NACHWEIS BIEGEKNICKEN DIN 18800 T2

Grundkomb. : Nachweis Biegeknicken nicht erforderlich.

NACHWEIS BIEGEDRILLKNICKEN DIN 18800 T2 (Ersatzstab nach BTII)

Grundkomb.	$N_d =$	0.00 kN	$M_{yd} =$	248.50	$M_{zd} =$	0.00 kNm
	$N_{kiz} =$	0.00 kN	$M_{kiy} =$	1176.63		kNm
Gleichung 16 : η	=	0.56	< 1			

DURCHBIEGUNGEN für 1-fache Lasten , $x_0 = 4.08 \text{ m}$ $z_{ul} w = L / 300$

vorh. w	/ $z_{ul} w$	=	2.50 / 2.72	=	0.92	< 1
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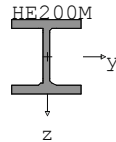
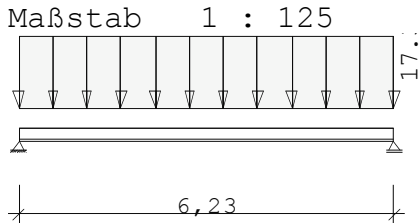
!!Vorbemessung!!

(Pos 13)

EINFELDTRÄGER HE 200 M

L = 6.23 m

S235



Teilsicherheitsbeiwert $\gamma_M = 1.10$ Eigengewicht $g = 1.030$ kN/m

EINFACHE LASTEN (q-Anteile : $\gamma_F = 1.50$; z_p : Oberseite)

Streckenlast 1 : $g_z = 17.30$ kN/m² $b_z = 1.00$ m

AUFLAGERKRÄFTE Grundkomb. (ohne γ_F) * = Maximum

Lager	Ew	V (kN)	Hx (kN)	Hy (kN)	aus Lasten **)
links	G	57.10	0.00	0.00	
rechts	G	57.10	0.00	0.00	

SPANNUNGEN Grundkomb. nach Th.1.O. , $x_0 = 3.12$ m γ_F -fach

vorh. σ_X / f_{yd}	=	124.12 / 218.2	=	0.57	< 1
vorh. τ / τ_{Rd}	=	0.00 / 126.0	=	0.00	< 1
vorh. σ_V / f_{yd}	=	124.12 / 218.2	=	0.57	< 1

NACHWEIS BIEGEKNICKEN DIN 18800 T2

Grundkomb. : Nachweis Biegeknicken nicht erforderlich.

NACHWEIS BIEGEDRILLKNICKEN DIN 18800 T2 (Ersatzstab nach BTII)

Grundkomb.	$N_d =$	0.00 kN	$M_{yd} =$	120.06	$M_{zd} =$	0.00 kNm
	$N_{kiz} =$	0.00 kN	$M_{kiy} =$	657.62		kNm
Gleichung 16 : η	=	0.51	< 1			

DURCHBIEGUNGEN für 1-fache Lasten , $x_0 = 3.12$ m $z_{ul} w = L / 300$

vorh. w / $z_{ul} w$	=	1.61 / 2.08	=	0.77	< 1
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!!Vorbemessung!!

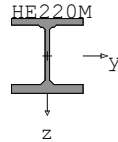
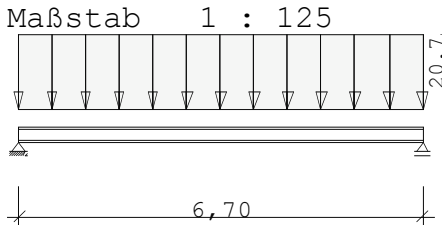
(Pos 16)

Deckenposition dann als Durchlaufträger.

EINFELDTRÄGER HE 220 M

L = 6.70 m

S235



Teilsicherheitsbeiwert $\gamma_M = 1.10$ Eigengewicht $g = 1.170 \text{ kN/m}$

EINFACHE LASTEN (q-Anteile : $\gamma_F = 1.50$; z_p : Oberseite)

Streckenlast 1 : $g_z = 20.75 \text{ kN/m}^2$ $b_z = 1.00 \text{ m}$

AUFLAGERKRÄFTE Grundkomb. (ohne γ_F) * = Maximum

Lager	E_w	V (kN)	H_x (kN)	H_y (kN)	aus Lasten **)
links	G	73.43	0.00	0.00	
rechts	G	73.43	0.00	0.00	

SPANNUNGEN Grundkomb. nach Th.1.O. , $x_0 = 3.35 \text{ m}$ γ_F -fach

vorh. σ_x / f_{yd}	=	136.48 / 218.2	=	0.63 < 1
vorh. τ / τ_{Rd}	=	0.00 / 126.0	=	0.00 < 1
vorh. σ_v / f_{yd}	=	136.48 / 218.2	=	0.63 < 1

NACHWEIS BIEGEKNICKEN DIN 18800 T2

Grundkomb. : Nachweis Biegeknicken nicht erforderlich.

NACHWEIS BIEGEDRILLKNICKEN DIN 18800 T2 (Ersatzstab nach BTII)

Grundkomb.	$N_d =$	0.00 kN	$M_{yd} =$	166.01	$M_{zd} =$	0.00 kNm
	$N_{kiz} =$	0.00 kN	$M_{kiy} =$	787.35		kNm
Gleichung 16 :	$\eta =$	0.56				< 1

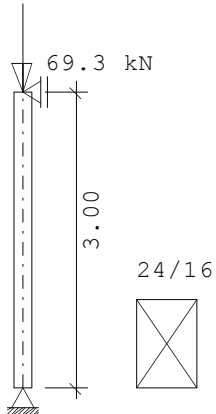
DURCHBIEGUNGEN für 1-fache Lasten , $x_0 = 3.35 \text{ m}$ zul $w = L / 300$

vorh. w / zul w	=	1.88 / 2.23	=	0.84 < 1
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Pos 24.1 Stütze

PENDELSTÜTZE: H = 3.00 m 24/16 cm

Nadelholz S 10



MASSGEBENDE SYSTEMGRÖSSEN für die Nachweise:

$$\begin{aligned} \text{sky} &= 300 \text{ cm} & \text{skz} &= 300 \text{ cm} \\ \text{Lambda} &= 65.0 & \text{Omega} &= 1.74 \end{aligned}$$

BELASTUNG: $V = 69.30 \text{ kN}$ LF (H)

SPANNUNGSNACHWEISE: für Max.-Werte

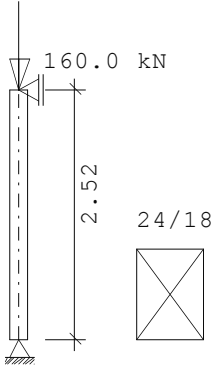
Zugseite	$\text{SigmaZ} = -1.80 \text{ MN/m}^2 < 7.00 \text{ MN/m}^2 (-0.26)$
Druck	$\text{SigmaD} = 1.80 \text{ MN/m}^2 < 8.50 \text{ MN/m}^2 (0.21)$
Knicknachweis	$\text{SigmaD} = 3.15 \text{ MN/m}^2 < 8.50 \text{ MN/m}^2 (0.37)$

Schwellendicke	Überstand	Aufstandslänge	Pressung [MN/m ²]
20.0 cm	20.0 cm	20.0 cm	1.80 < 2.00 (0.90)

Pos 24.2 Stütze

PENDELSTÜTZE: H = 2.52 m 24/18 cm

Nadelholz S 10



MASSGEBENDE SYSTEMGRÖSSEN für die Nachweise:

sky	=	252 cm	skz	=	252 cm
Lambda	=	48.5	Omega	=	1.39

BELASTUNG: V = 160.00 kN LF (H)

SPANNUNGSNACHWEISE: für Max.-Werte

Zugseite	SigmaZ =	-3.70 MN/m2	<	7.00 MN/m2	(-0.53)
Druck	SigmaD =	3.70 MN/m2	<	8.50 MN/m2	(0.44)
Knicknachweis	SigmaD =	5.14 MN/m2	<	8.50 MN/m2	(0.60)

Schwellendicke	Überstand	Aufstandslänge	Pressung [MN/m2]
14.0 cm	20.0 cm	20.0 cm	3.70 > 2.00 (1.85)

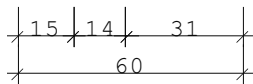
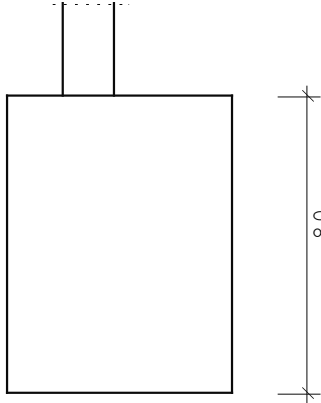
Schwellenlagerung nicht möglich!

Gewählt Stahlschuh 180/240mm mit 2 Bolzen M 12 zur Lagesicherung. Verankerung in Bodenplatte bzw. Streifenfundamente.

Fundament

Pos 30 Streifenfundament

Maßstab 1 : 20



Streifenfundament 60 / 80 C 20/25 BSt 500 S(B)

Abmessungen

Wand	d =	0.14 m	L =	1.00 m	Mauerwerk
Streifenfundament	b =	0.60 m	L =	1.00 m	d = 0.80 m
Bewehrungslage	d1 =	5.0 cm			

Überstand	links =	0.15 m	re =	0.31 m
vorgeg.Überstand	links =	0.15 m	re =	beliebig

Belastung

Vertikalkräfte	G =	100.00 kN	g =	100.00 kN/m
	P =	0.00 kN	p =	0.00 kN/m
Fundament	G1 =	16.20 kN	g1 =	16.20 kN/m
	Q =	116.20 kN	q =	116.20 kN/m

BODENPRESSUNGEN : ohne klaffende Fuge

Bodenpressung nach DIN 1054	Sigma =	245.00 kN/m ²
Kantenpressungen : max Sigma Li	=	320.00 kN/m ²
max Sigma Re	=	53.33 kN/m ²

Bemessung für 1.35-fache g- und 1.5-fache p-Lasten :

nach Heft 240 Punkt 2.5
keine Biegebewehrung erforderlich

!!Vorbemessung!!

Schubbemessung C 20/25 BSt 500 S(B) nach DIN 1045-1

links: $V_{Ed} = -355.20 \text{ kN/m} < V_{Rd,ct} = 41.92 \text{ kN/m}$

$aswQ^* = 7.07 \text{ cm}^2/\text{m je lfd. m}$

* Mindestschubbewehrung

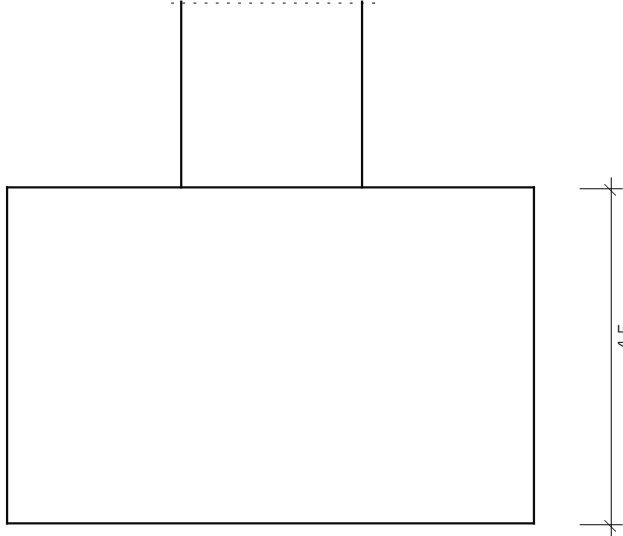
rechts: $V_{Ed} = 35.20 \text{ kN/m} < V_{Rd,ct} = 41.92 \text{ kN/m}$

$aswQ^* = 7.07 \text{ cm}^2/\text{m je lfd. m}$

* Mindestschubbewehrung

Pos 30.1 Streifenfundament

Maßstab 1 : 10



Streifenfundament 70 / 45 C 20/25 BSt 500 S (B)

Abmessungen

Wand	d =	0.24 m	L =	1.00 m	Mauerwerk
Streifenfundament	b =	0.70 m	L =	1.00 m	d = 0.45 m
Bewehrungslage	d1 =	5.0 cm			

Belastung

Vertikalkräfte	G =	150.00 kN	g =	150.00 kN/m
	P =	0.00 kN	p =	0.00 kN/m
Fundament	G1 =	10.63 kN	g1 =	10.63 kN/m
	Q =	160.63 kN	q =	160.63 kN/m

zentrische Pressung $p = 225.54 \text{ kN/m}^2$

Bemessung für 1.35-fache g- und 1.5-fache p-Lasten :

nach Heft 240 Punkt 2.5

keine Biegebewehrung erforderlich

Schubbemessung C 20/25 BSt 500 S (B) nach DIN 1045-1

links: $V_{Ed} = -49.85 \text{ kN/m} < V_{Rd,ct} = 46.87 \text{ kN/m}$

$asw_{Q^*} = 7.07 \text{ cm}^2/\text{m je lfd. m}$

* Mindestschubbewehrung

rechts: $V_{Ed} = -49.85 \text{ kN/m} < V_{Rd,ct} = 46.87 \text{ kN/m}$

$asw_{Q^*} = 7.07 \text{ cm}^2/\text{m je lfd. m}$

* Mindestschubbewehrung

Objektdaten

Bauherr: Maharishi Weltfriedens Stiftung
Bauvorhaben: Seminargebäude
Bauort: Deutschland
Straße:
Gemarkung / Flur / Objekt-Nr.

Verwendetes Verfahren

EnEV - Monatsbilanzverfahren

Gebäudevolumen brutto V_e	6509,03 m ³	Beheiztes Luftvolumen V	5207,22 m ³
Gebäudenutzfläche A_N	2082,89 m ²		
Luftdichtheit Gebäudehülle	nicht geprüft	Luftwechselzahl n	0,7 / h
Heizunterbrechung	10 h/d	Interne Lasten	6 W/m ²
Gebäudeheizlast DIN 4108-6	82,38 kW		
Klimaregion	Mittlerer Standort		
Wärmebrücken	pauschal nach DIN 4108 Beiblatt 2		
Bauart	leicht, wirksame Speicherfähigkeit pauschal 15 Wh/(m ³ *K)*V _e		
Volumenermittlung V _e	22,86*36,06*7,85+(4*4*1,50)+((1,72/3)*(16+4,47+4))		

EnEV-Anforderungsprofil

Gebäudetyp Nicht-Wohngebäude
Wärmebereitstellung Fossil oder elektrisch konventionell

Technische Abteilung der Maharishi Veda GmbH

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Liste der verwendeten Bauteile:

Außenwand Faserdämmung

Richtung **N ↑**

Fläche **71,30 m²** F_x **1,00**

Flächenberechnung

$22,86 \cdot 7,85 + 4,00 \cdot 2,00 - (2 \cdot 8 + 14,54) \cdot 2 - 1 \cdot 3,5 - 22,54 - 29,03$

Schichtaufbau	λ [W/mK]	Dicke [cm]
---------------	------------------	------------

R_{se} **0,04 m²K/W**

Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
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Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
--	-------	------

Faserdämmstoffe nach DIN 18165-1 035	0,035	14,00
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OSB-Platten	0,130	1,80
-------------	-------	------

Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
--------------------------------------	-------	------

PA-Folie ≥ 0.05 mm		0,01
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Wandbauplatten aus Gips 18163	0,290	1,250
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R_{si} **0,13 m²K/W**

U-Wert **0,16 W/(m²K)** α -Wert **0,40**

Außenwand Faserdämmung

Richtung **S ↓**

Fläche **71,30 m²** F_x **1,00**

Flächenberechnung

$22,86 \cdot 7,85 + 4,00 \cdot 2,00 - (2 \cdot 8 + 14,54) \cdot 2 - 1 \cdot 3,5 - 22,54 - 29,03$

Schichtaufbau	λ [W/mK]	Dicke [cm]
---------------	------------------	------------

R_{se} **0,04 m²K/W**

Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
---	-------	------

Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
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Faserdämmstoffe nach DIN 18165-1 035	0,035	14,00
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OSB-Platten	0,130	1,80
-------------	-------	------

Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
--------------------------------------	-------	------

PA-Folie ≥ 0.05 mm		0,01
-------------------------	--	------

Wandbauplatten aus Gips 18163	0,290	1,250
-------------------------------	-------	-------

R_{si} **0,13 m²K/W**

U-Wert **0,16 W/(m²K)** α -Wert **0,40**

Technische Abteilung der Maharishi Veda GmbH

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Außenwand Faserdämmung

Richtung **O →**

Fläche **127,82 m²** F_x **1,00**

Flächenberechnung

$36,06 \cdot 7,85 + 4,00 \cdot 2,00 - (2 \cdot 10 + 4,072 \cdot 4) \cdot 2 - 2,5 - 4,072 - 1 \cdot 3,50 - 35,73 - 4,87$

Schichtaufbau	λ [W/mK]	Dicke [cm]
---------------	------------------	------------

R_{se} **0,04 m²K/W**

Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Faserdämmstoffe nach DIN 18165-1 035	0,035	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie $\geq 0,05$ mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,16 W/(m²K)** α -Wert **0,40**

Außenwand Faserdämmung

Richtung **W ←**

Fläche **122,15 m²** F_x **1,00**

Flächenberechnung

$36,06 \cdot 7,85 + 4,00 \cdot 2,00 - (2 \cdot 14 + 4,072 \cdot 3) \cdot 2 - 1 \cdot 3,50 - 40,12 - 44,87$

Schichtaufbau	λ [W/mK]	Dicke [cm]
---------------	------------------	------------

R_{se} **0,04 m²K/W**

Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Faserdämmstoffe nach DIN 18165-1 035	0,035	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie $\geq 0,05$ mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,16 W/(m²K)** α -Wert **0,40**

Technische Abteilung der Maharishi Veda GmbH

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Außenwand Ständer

Richtung **N ↑**

Fläche **22,54 m²** F_x **1,00**

Flächenberechnung

$$20 \cdot (0,14 \cdot 7,85) + 2 \cdot (0,14 \cdot 2,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Außenwand Ständer

Richtung **S ↓**

Fläche **22,54 m²** F_x **1,00**

Flächenberechnung

$$20 \cdot (0,14 \cdot 7,85) + 2 \cdot (0,14 \cdot 2,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Außenwand Ständer

Richtung **O →**

Fläche **35,73 m²** F_x **1,00**

Flächenberechnung

$$32 \cdot (0,14 \cdot 7,85) + 2 \cdot (0,14 \cdot 2,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

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Außenwand Ständer

Richtung **W ←**

Fläche **40,12 m²** F_x **1,00**

Flächenberechnung

$$36 \cdot (0,14 \cdot 7,85) + 2 \cdot (0,14 \cdot 2,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Außenwand Riegel

Richtung **N ↑**

Fläche **29,03 m²** F_x **1,00**

Flächenberechnung

$$6 \cdot (0,2 \cdot 22,86) + 2 \cdot (0,2 \cdot 4,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Außenwand Riegel

Richtung **S ↓**

Fläche **29,03 m²** F_x **1,00**

Flächenberechnung

$$6 \cdot (0,2 \cdot 22,86) + 2 \cdot (0,2 \cdot 4,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

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Außenwand Riegel

Richtung **O →**

Fläche **44,87 m²** F_x **1,00**

Flächenberechnung

$$6 \cdot (0,2 \cdot 36,06) + 2 \cdot (0,2 \cdot 4,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Außenwand Riegel

Richtung **W ←**

Fläche **44,87 m²** F_x **1,00**

Flächenberechnung

$$6 \cdot (0,2 \cdot 36,06) + 2 \cdot (0,2 \cdot 4,00)$$

Schichtaufbau	λ [W/mK]	Dicke [cm]
R_{se} 0,04 m²K/W		
Putzmörtel aus Kalk, Kalkzement, hydr. Kalk	1,000	2,50
Holzwohle-Leichtbauplatten nach DIN 1101 065	0,065	5,00
Konstruktionsholz	0,180	14,00
OSB-Platten	0,130	1,80
Faserdämmstoffe nach DIN 18165-1 050	0,050	5,00
PA-Folie ≥ 0.05 mm		0,01
Wandbauplatten aus Gips 18163	0,290	1,250

R_{si} **0,13 m²K/W**

U-Wert **0,34 W/(m²K)** α -Wert **0,40**

Dach an Außenluft

Richtung **O →** Neigung **0°**

Fläche **824,33 m²** F_x **1,00**

Flächenberechnung

$$36,06 \cdot 22,86$$

R_{se} 0,04 m²K/W		
Kunststoff-Dachbahn DIN 16729		0,50
Polyurethan(PUR)-Hartschaum 035	0,035	24,00
PTFE-Folie ≥ 0.05 mm		0,01
OSB-Platten	0,130	2,20

R_{si} **0,13 m²K/W**

U-Wert **0,14 W/(m²K)** α -Wert **0,80**

Bemerkungen

Dachneigung $>1\%$

Technische Abteilung der Maharishi Veda GmbH

Hannover

Bodenplatte

Fläche **824,33 m²** F_x **0,35**

Flächenberechnung

36,06*22,86

Schichtaufbau λ [W/mK] Dicke [cm]

Umfang der Bodenplatte: 117,8 m

Ohne Grundwasser
Ohne Randdämmung

R_{se} **0,00 m²K/W**

Beton nach DIN 206, mittlere Rohdichte 1,350 16,00

Polystyrol-Partikelschaum DIN 18164 040 0,040 10,00

Anhydrit-Estrich 1,200 6,00

Linoleum 0,170 1,00

R_{si} **0,17 m²K/W**

U-Wert **0,35 W/(m²K)**

Fenster allgemein

Richtung **N ↑**

Fläche **64,58 m²** F_x **1,00**

Flächenberechnung

F_s **0,90** F_c **1,00** F_F **0,70** g-Wert **0,60**

(2*8+14,54)*2+3,5

U-Wert **1,30 W/(m²K)**

Bemerkungen

Zweifachverglasung $U_g = 1,1$ Rahmen m. Prüfzeugnis

Fenster allgemein

Richtung **S ↓**

Fläche **64,58 m²** F_x **1,00**

Flächenberechnung

F_s **0,90** F_c **1,00** F_F **0,70** g-Wert **0,60**

(2*8+14,54)*2+3,5

U-Wert **1,30 W/(m²K)**

Bemerkungen

Zweifachverglasung $U_g = 1,1$ Rahmen m. Prüfzeugnis

Fenster allgemein

Richtung **O →**

Fläche **80,15 m²** F_x **1,00**

Flächenberechnung

F_s **0,90** F_c **1,00** F_F **0,70** g-Wert **0,60**

(2*10+4,072*4)*2+4,072+3,5

U-Wert **1,30 W/(m²K)**

Bemerkungen

Zweifachverglasung $U_g = 1,1$ Rahmen m. Prüfzeugnis

Technische Abteilung der Maharishi Veda GmbH

Hannover

Fenster allgemein

Richtung **W ←**

Fläche **83,93** m² F_x **1,00** Flächenberechnung
 F_s **0,90** F_c **1,00** F_F **0,70** g-Wert **0,60** (2*14+4,072*3)*2+3,5
U-Wert **1,30** W/(m²*K)

Bemerkungen

Zweifachverglasung Ug = 1,1 Rahmen m. Prüfzeugnis

Außentür opak

Richtung **O →**

Fläche **2,50** m² F_x **1,00** Flächenberechnung
2,5

U-Wert **1,70** W/(m²*K) α -Wert **0,40**

Bemerkungen

Vollholztür, schwere Ausführung, Dicke des Türblattes ca. 60 mm

Technische Abteilung der Maharishi Veda GmbH

Hannover

Ziegel - EnEV Ergebnisreport Monatsbilanz

	Jan	Feb	Mrz	Apr	Mai	Jun	Jul	Aug	Sep	Okt	Nov	Dez	
Q_t	11418	9348,1	8381,0	5171,2	3431,2	1796,3	562,5	393,7	2504,0	5568,6	7784,1	9956,0	kWh
Q_{WB}	1967,7	1610,9	1444,3	891,1	591,3	309,6	96,9	67,9	431,5	959,6	1341,4	1715,7	kWh
Q_{FH}	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	kWh
Q_{sop}	-87,0	-14,1	96,3	492,8	573,8	706,6	736,6	460,5	293,3	74,7	-64,1	-138,5	kWh
Q_V	18718	15324	13739	8476,9	5624,5	2944,6	922,1	645,4	4104,6	9128,3	12760	16320	kWh
Q_{II}	2960,3	2261,5	1810,8	972,7	610,9	317,7	99,5	69,6	442,8	1056,5	1652,5	2351,3	kWh
ΣQ_I	29231	24036	21657	13074	8462,3	4026,2	745,4	576,9	6303,9	14525	20297	25779	kWh
Q_s	2182,5	2628,1	4064,5	8203,3	8709,6	9651,0	10320	7750,9	6195,5	3981,5	2264,3	1325,8	kWh
Q_{TWD}	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	kWh
Q_{Wiga}	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	kWh
Q_i	9298,0	8398,2	9298,0	8998,1	9298,0	8998,1	9298,0	9298,0	8998,1	9298,0	8998,1	9298,0	kWh
ΣQ_g	11480	11026	13363	17201	18008	18649	19618	17049	15194	13279	11262	10624	kWh
γ	0,39	0,46	0,62	1,32	2,13	4,63	26,32	29,55	2,41	0,91	0,55	0,41	-
η_g	0,984	0,973	0,935	0,675	0,456	0,215	0,038	0,034	0,407	0,829	0,952	0,981	-
Q_h	17939	13308	9166,8	1471,3	247,4	8,4	0,0	0,0	124,2	3520,5	9578,4	15360	kWh

Ziegel - EnEV Ergebnisreport

Hüllfläche	A	2605,70	m ²
Bezugsfläche	A _N	2082,89	m ²
Bruttovolumen	V _e	6509,03	m ³
Hüllflächenfaktor	A/V _e	0,40	1/m
Fensterflächenanteil	f _s	0,16	-
Nutzbare interne Gewinne	Q' _i	10,44	kWh / (m ³ a)
Nutzbare Solargewinne Fenster	Q' _s	4,61	kWh / (m ³ a)
Nutzbare Solargewinne Glasvorbau	Q' _{ss}	0,00	kWh / (m ³ a)
Nutzbare Solargewinne TWD	Q' _{TWD}	0,00	kWh / (m ³ a)
Nutzbare Gesamtgewinne	Q' _g	15,05	kWh / (m ³ a)
Lüftungswärmeverluste	Q' _v	16,70	kWh / (m ³ a)
Transmissionswärmeverluste	Q' _T	9,71	kWh / (m ³ a)
Wärmebrückenverluste	Q' _{WB}	1,76	kWh / (m ³ a)
Reduzierung durch Nachtabenkung	Q' _{il}	2,24	kWh / (m ³ a)
Flächenbez. Transmissionswärmeverlust vorh.	H' _{T,vorh.}	0,34	W / (m ² K)
Flächenbez. Transmissionswärmeverlust zul.	H' _{T,zul.}	0,68	W / (m ² K)
Heiztage	t _{HP}	201	d
Heizwärmebedarf	Q' _h	10,87	kWh / (m ³ a)
Trinkwasser-Wärmebedarf	Q' _{TW}	0,00	kWh / (m ³ a)
Gesamt-Aufwandszahl	e _P	1,52	-
Primärenergiebedarf vorh.	Q' _{vorh.}	16,52	kWh / (m ³ a)
Primärenergiebedarf zul.	Q' _{zul.}	19,54	kWh / (m ³ a)

Nachweis erbracht

Name _____

Datum _____

Funktion/
Firma _____

Unterschrift _____

Anschrift _____

Stempel / Firmenzeichen

Technische Abteilung der Maharishi Veda GmbH

Hannover

Nachweis über energiesparenden Wärmeschutz und energiesparende Anlagentechnik bei Gebäuden gemäß Energieeinsparverordnung 2002 / 2003

Bauherr: Maharishi Weltfriedens Stiftung

Bauvorhaben: Seminargebäude

Bauort: Deutschland

Straße:

Gemarkung / Flur / Objekt-Nr.

Die "Verordnung über energiesparenden Wärmeschutz und energiesparende Anlagentechnik bei Gebäuden (Energieeinsparverordnung - EnEV)" vom 1.2.2002

Grundlagen : DIN 4108-2
DIN V 4108-6
DIN V 4701-10 und Beiblatt 1
DIN EN ISO 6946
Bauaufsichtliche Zulassungen und Bescheide

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Nachweis erstellt mit Programmversion 4.10

Dateiname: SEMINARGEBAUDE03-05-07

Druckdatum: 09.05.07 - 16:17